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> Date: March 26, 2024 Project #: 2401801

To: Jack Greenburg The Brock Zents Partnership 181 Eglington Ave E, Suite #204 Toronto, ON M4P 1J4

From: Tanvi Patel M.Env.Sc., G.I.T. and Jason Cole, M.Sc., P. Geo.

Re: Supplemental Hydrogeological Assessment: Short-Term Dewatering and Long-Term Seepage Estimate Updates 2660-2680 Brock Road, Pickering, ON

1. Introduction

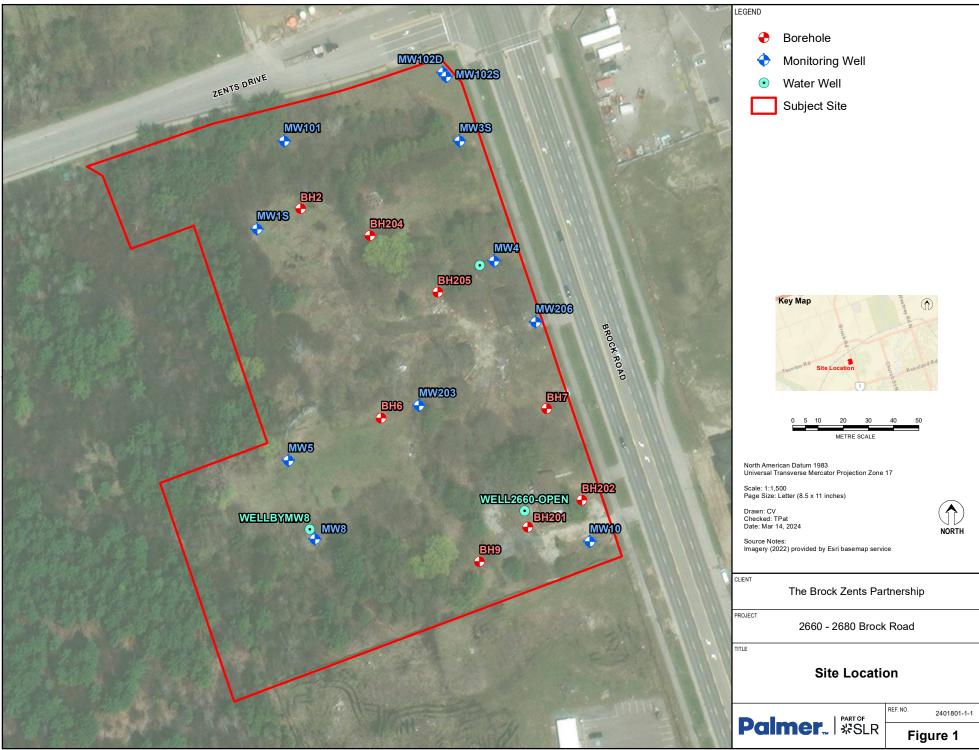
Palmer was retained by The Brock Zents Partnership (Brock Zents or the "client") to undertake a Supplemental Hydrogeological Assessment to provide updated short-term dewatering and long-term groundwater seepage rate estimates for the proposed development located at 2660-2680 Brock Road, Pickering Ontario (the "site"). The site is located on the southwest corner of Brock Road and Zents Road (**Figure 1**).

Based on the site plan provided by GMB Architecture (2023) (**Attachment A**), the proposed development will consist of 274 unit Stacked Townhouse Development with one (1) level of basement garage parking. **Table 1** summarizes the P1 depth, anticipated excavation depths and dimensions of each parking level.

Parking Section	Ground Surface Elevation (masl)	P1 Elevation (masl)	Approx. Anticipated Excavation Elevation (masl)	Approx. Excavation Length (m)	Approx. Excavation Width (m)
Brock Road	129.9 - 130.7	128.25 – 128.95	126.8 – 127.6	159.8	17.8
Zents Drive	130.7 – 131.2	129.35 – 130.55	127.9 – 129.3	92.6	17.8

Note: metres above sea level (masl)

Based on the results of previously hydrogeological reporting by Terrapex (2022), the P1 elevations presented in **Table 1** have been raised from their original elevation of approximately 126 masl to avoid a confined aquifer identified below the site.



Document Path: G:\Shared drives\Projects 2024\24018 - The Brock Zents Partnership\2401801- 2660-2680 Brock Road HydroG\GIS\1_Workspace\Task 1 - HydroG Figures\2401801-1-1-Site Location.mxd

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2. Existing Conditions Summary

The site is located within the Iroquois Plain physiographic region, south of South Slope physiographic region (Chapman & Putnam, 1984). Ontario Geological Survey (OGS) Surficial Geology of Southern Ontario (2010) mapping indicated that the site is situated in an area composed of course-textured glaciolacustrine deposits overlying Newmarket Till. Based on local knowledge, it is anticipated that the Thorncliffe Aquifer may be present at shallow depths below the Newmarket Till in the vicinity of the site.

3. Site Specific Geology

3.1 Site Stratigraphy

The stratigraphy underlaying site is delineated based on boreholes and monitoring wells drilled for a previous assessment completed as part of Terrapex's Hydrogeological Review (Terrapex, 2022). Borehole logs are included for reference in **Attachment B**. The monitoring wells were constructed using environmental grade, 50 mm diameter, Schedule 40, PVC piping with machine slotted (10 slot) screens at the bottom that were open for a 1.5 m or 3.0 m length.

The observed stratigraphy on site generally comprised of topsoil and fill overlying thin coarse-textured glaciolacustrine sediments underlain by clayey, sandy silt Newmarket Till. Below the Newmarket Till, deposits of silt, sandy silt, sand, gravelly sand to sandy gravel were found below the Newmarket Till and are expected to represent the Thorncliffe Aquifer. Cross Sections from Terrapex (2024) are included in **Attachment C**, that shows the site hydrostratigraphy.

3.2 Groundwater Level and Flow

Static groundwater levels were previously measured by Terrapex between May 2018 and October 2021. Groundwater levels were measured by Palmer on March 6th, 2024. Groundwater levels measured in fourteen (14) monitoring wells ranged from 0.12 to 7.97 mbgs (123.6 to 131.0 masl). **Table 2** below summarizes groundwater level data collected by both Terrapex and Palmer to show the seasonal high, seasonal low and a comparison between the Palmer and Terrapex data. The full suite of previous groundwater level data from Terrapex is presented in **Attachment D**.

Surface			G	W Levels ·	GW Levels – Palmer					
MW ID	Elevation	Date Ranges for		mbgs		masl		06-March-2024		
	(masl)	Terrapex Data	Deep	Shallow	Deep	Shallow	mbgs	masl		
MW1(S)	132.0	May 2018 - Oct 2021	1.1	0.19	130.93	131.84	0.45	131.6		
MW1(D)	132.0	May 2018 - Oct 2021	5.28	3.96	126.75	128.07	5.00	127.0		
MW3(D)	130.4	May 2018 - Oct 2021	5.34	2.23	125.04	128.14	3.56	126.8		
MW3(S)	130.3	May 2018 - Oct 2021	> 3.10	1.2	<127.24	129.15	2.86	127.5		
MW4	129.8	May 2018 - Oct 2021	4.88	2.07	124.9	127.7	3.13	126.6		
MW5	131.6	May 2018 - Oct 2021	7.97	5.11	123.6	126.5	6.26	125.3		
MW8(D)	131.6	May 2018 - Oct 2021	7.37	4.64	124.3	127.0	5.85	125.8		

Table 2. Groundwater Level Flow Summary

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Surface MW ID Elevation			G	W Levels ·	GW Levels – Palmer			
		Date Ranges for	mbgs		masl		06-March-2024	
	(masl)	Terrapex Data	Deep	Shallow	Deep	Shallow	mbgs	masl
MW8(S)	131.0	Jun 2019 - Oct 2021	> 3.96	1.49	<127.07	129.6	0.67	130.4
MW10	129.3	May 2018 - Oct 2021	> 3.90	0.35	<125.49	128.9	0.63	128.7
MW101	131.2	Jun 2019 - Oct 2021	2.56	0.29	128.7	131.0	0.12	131.1
MW102(D)	130.7	Jun 2019 - Oct 2021	5.66	2.88	125.0	127.8	3.76	126.9
MW102(S)	130.7	Jun 2019 - Oct 2021	> 3.66	2.44	<127.02	128.2	3.19	127.5
MW203	131.6	Oct 2021	> 6.10	5.01	<125.51	125.7	3.11	128.5
MW206	130.6	Oct 2021	3.75	3.65	125.94	126.0	2.95	127.61

3.3 Single Well Response Testing

On March 6th, 2024, Palmer staff conducted single wells response testing at select pre-existing monitoring wells to assess the in-situ hydraulic conductivity of the screened stratigraphy. A change in head was created in each of the five (5) wells through removing a column of groundwater and the rate of recovery was measured using a datalogger installed in the well to collect groundwater level measurements during the test.

Hydraulic conductivity values (K-value) were then estimated using the displacement-time data and were analysed using the Unconfined Bouwer-Rice (1976) method using AqtesolvTM software. The analysis results are presented in **Attachment E**, and the range of estimated K-values are summarized below in **Table 3**. The hydraulic conductivity results collected by Palmer are consistent with the hydraulic conductivity results as reported by Terrapex (2022).

MW ID	Screened Geology	Solution	Test Type	Hydraulic Conductivity (K) (m/s)	Geometric Mean K (m/s)	90 th Percentile K (m/s)
MW1(S)	-	Unconfined Bouwer-Rice	Rising Head	1.4x10 ⁻⁸	4.0.40.8	4.0.40.8
MW101	Till	Unconfined Bouwer-Rice	Rising Head	1.9x10 ⁻⁸	1.6x10 ⁻⁸	1.8x10 ⁻⁸
MW3(D)	Aquifer: Dense sandy	Unconfined Bouwer-Rice	Rising Head	1.3x10 ⁻⁵	-	-
MW4	silt / silty sand	Unconfined Bouwer-Rice	Rising Head	7.6x10 ⁻⁸	-	-

Table 3. Hydraulic Conductivity Summary Table

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4. Short-Term Dewatering and Long-Term Seepage Assessment

Based on our understating of the project, the proposed development includes the installation of two underground parking garages to a P1 level (**Attachment A**). The proposed dimensions and elevations of the parking garage level P1 for both the Zents Drive and Brock Road parking garages are summarized in **Table 1**.

Palmer. SLR

As previously mentioned, the P1 foundation level has been raised from previous designs to avoid intercepting the Thorncliffe Aquifer (shown in cross section in **Appendix C**). Based on the borehole logs and soil permeability data reviewed by Palmer, it is interpreted that the new P1 foundation level is located entirely within the till and thin glaciolacustrine soils, sufficiently above the Thorncliffe Aquifer. Assuming that footings will need to be dug to 1.5 m below the P1 level, an area near the foundation for Block 4 (at MW4) is interpreted to intercept a small portion of the Thorncliffe Aquifer. Additional dewatering should be expected in this area and have been included in our short-term dewatering calculations. The remainder of the excavations for the P1 level are expected to terminate within the till soils.

It is important to note that all excavation dimensions and depths used in this report are presented solely for the purposes of estimating groundwater dewatering rates and are not intended to direct construction activity.

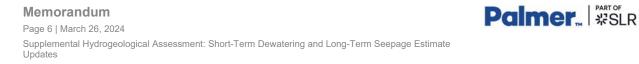
4.1 Short-Term Construction Dewatering

4.1.1 Dewatering Rate Estimate

To maintain stable and dry working conditions within till soils, the water level should be lowered to at least 0.5 m below the proposed maximum excavation depth. The excavation depth and dimension for both parking garages are outlined in **Table 1**. To account for footing depth and excavation, 1.5 m was added to the Parking Level (P1) depths for both parking garages. The spring months highest measured groundwater level during the previous monitoring period was used to account for springtime high groundwater levels.

The highest measured groundwater level during a previous monitoring event on April 23rd, 2019, is 131.8 masl (at MW1(S)). The highest measured groundwater level underlying the extent of the Brock Road parking garage during a previous monitoring event on May 17th, 2018, is 129.15 masl (MW3(S)). These values are interpreted represent a perched groundwater table within the upper till and thin glaciolacustrine soils that is perched on the low permeability till below. The true groundwater table is interpreted to be located within the Thorncliffe Aquifer as represented by water level data from MW3D, MW4, MW102D, MW106.

Dewatering rate estimate (Q) for the building foundation excavations was calculated using trench shaped unconfined solution. The calculation used the highest groundwater level measured underlying each parking garage, deepest foundation elevations, and the highest K-value calculated for dewatered soils. A dewatering rate estimate is also provided for a ~30 m section of Brock Rd parking garage where there is expected to be higher permeability soils to give a conservative value to account for the potential of encountering sandy/silty sand/sandy silt soils in the Thorncliffe Aquifer, which would result in higher



dewatering rates over this length. Dewatering calculations assume the entire parking garage extent will be constructed at the same time.

Table 4 summarizes the dewatering rate estimates (Q) calculations for the two (2) open-cut parking garages using the following equation from Powers et.al (2007) for trench-shaped excavations:

Unconfined solution:

$$Q_{open \, cut} = Q = \frac{\pi K (H^2 - h^2)}{\ln \left(\frac{Ro}{r_e}\right)} + 2 \left[\frac{x K (H^2 - h^2)}{2L}\right] \qquad m^3 / s$$

Where	К	=	hydraulic conductivity (m/s)
	Н	=	saturated thickness (m)
	h	=	saturated thickness after dewatering (m)
	R ₀	=	radius of influence estimated using the Sichardt Approximation: $R = 3000 * (H-h)^* \sqrt{K}$ (m)
	<i>r</i> e	=	equivalent well radius estimated by: $r_e = \sqrt{\frac{a * x}{\pi}}$ (m) <i>Where</i> $a = excavation width (m)$ x = excavation length (m)

Table 4. Dewatering Rate Assessment

Parameters	Zents Drive Parking Garage	Brock Road Parking Garage						
Parking Level 1 Base Elevation (masl)	129.35	128.25						
Approx. Excavation Elevation (masl)	127.90	126.80						
Groundwater Level Target (masl)	126.90	125.80						
Assumed High Groundwater Level (masl)	131.8	129.2						
Dewatered Medium	Till	Till	Aquifer Sand/Silty Sand / Sandy Silt					
K (m/s)	1.9x10 ⁻⁸	1.9x10 ⁻⁸	9.8x10 ⁻⁶					
H (m)	4.40	2.90	14					
h (m)	0	0	13					
a (m)	17.8	17.8	17.8					
x (m)	92.6	159.8	30.0					
R₀ (m)	24.7	31.3	22.4					

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Parameters	Zents Drive Parking Garage	Brock Road Parking Garage					
Q (L/day)	1,544	1,251	193,541				
Q UF=1.5x (L/day)	2,316	292,191					

a – trench width

x – trench length

K-hydraulic conductivity of dewatered medium

H – initial saturated thickness h – saturated thickness after dewatering

n – saturated thickness

R – radius of influence L – line source distance

L – line source distance

Q – dewatering rate

As summarized in **Table 4**, the groundwater dewatering rate expected for the Zents Drive parking garage is 2,316 L/day. To be conservative, the Contractor should expect a rate of approximately 5,000 L/day of groundwater seepage into the Zents Drive parking garage. A 25 mm storm event could add an additional 41,207 L to the Zents Drive parking structure excavation.

As also summarized in **Table 4**, the groundwater dewatering rate expected for the Brock Road parking garage is 292,191L/day. Excavations in the till soils are expected to be similar to those along Zents Drive, but due to an approximately 30 m long section of the Brock Road parking garage where more permeable sands are expected to bisect the excavation depth, there is expected to be a section of the excavation near Block 4 where additional dewatering effort will be required. A 25 mm storm event could add an additional 71,111 L to the Brock Road parking structure excavation.

4.2 Foundation Drainage Assessment

Following completion of the P1 foundation excavation, it is expected that the P1 invert level (shown as the P1 Driving Level in **Appendix C**) will be generally located above the groundwater table. Accounting for potentially higher spring water levels and the presence of a foundation water collection drain located at 0.3 m below the P1 level, there appears to be a potential for some minor groundwater seepage into the foundation drains. The P1 foundation collection drains are located above the Thorncliffe Aquifer and any seepage is interpreted to originate from the till and glaciolacustrine soils.

Table 5 summarises the estimated results for the rate of long-term groundwater seepage into the foundation drains of the Zents Ave and Brock Road underground parking structures. Based on this assessment, approximately 941 L/day of groundwater seepage is expected for the Zents Drive foundation and 5,280 L/day of groundwater seepage is expected for the Brock Road foundation.

Parking Section	Ground Surface Elevation (masl)	P1 Elevation (masl)	Approx. Anticipated Excavation Elevation (masl)	Approx. Excavation Length (m)	Approx. Excavation Width (m)	
Brock Road	129.9 - 130.7	128.25 – 128.95	126.8 – 127.6	159.8	17.8	
Zents Drive	130.7 – 131.2	129.35 – 130.55	127.9 – 129.3	92.6	17.8	

Paran	neters	Zents Drive Parking Garage	Brock Road Parking Garage		
Horizontal	Width (m)	17.8	17.8		
Dimension	Length (m)	92.6	159.8		
Footing Elev	vation (masl)	129.2	128.1		
Groundwater Lev	vel Target (masl)	128.6	127.5		
Groundwater	Level (masl)	131.8	129.2		
Dewatere	d Medium	Till	Till and Glaciolacustrine Soils		
K (m/s) -	geomean	1.8x10 ⁻⁸	3.9x10 ⁻⁷		
H ((m)	3.2	1.7		
h (m)	0	0		
a (m)		17.8	17.8		
x (m)	92.6	159.8		
Q _{drainage} (L/day)		976	4,028		

a – trench width

x - trench length

K –hydraulic conductivity of dewatered medium

H - initial saturated thickness

h - saturated thickness after dewatering

R – radius of influence

L – line source distance

Q – dewatering rate

4.3 Water Taking Permit Recommendations

Construction dewatering in excess of 50,000 L/day requires a registration on the Environmental and Section Registry (EASR) with the Ministry of the Environment, Conservation and Parks (MECP). Dewatering above 400,000 L/day requires a Category 3 Permit to Take Water (PTTW).

Based on the dewatering rates provided above, the highest dewatering rate based on the assumptions provided is estimated to be 292,191 L/day Brock Road P1 Parking Garage excavation to account for the potential of encountering permeable silty sand/sandy silt soils of the Thorncliffe Aquifer. An additional 5,000 L/day of construction dewatering should be expected for the Zents Drive P1 Parking Garage excavation. Projects with overlapping groundwater drawdown cones are required to be permitted under the same EASR or PTTW.

Therefore, an EASR registration with the MECP is required for construction phase dewatering in the amount of 297,191 L/day.

As the long-term groundwater seepage is less than 50,000 L/day, no MECP permitting is required.

4.4 Sewer Discharge Permitting Recommendations

A temporary sewer discharge permit should be obtained from Durham Region for dewatering discharge into the storm sewer along Brock Road. Additional groundwater quality sampling against Durham Region



Storm Sewer Discharge By-Law Criteria will be required as part of this permit submission. A SWM Engineer should confirm that the sewer has the capacity to handle the proposed construction phase dewatering volumes.

Given the low long-term seepage rates predicted, it is expected that the storm sewer will have capacity for the volume of foundation drainage water; however, this should be confirmed with the SWM Engineer for the project and/or Durham Region.

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5. Statement of Limitations

The extent of this study was limited to the specific scope of work for which we were retained and that is described in this report. Palmer has assumed that the information provided by the client, or any secondary sources of information are factual and accurate. Palmer accepts no responsibility for any deficiency, misstatement or inaccuracy contained in this report as a result of omissions, misinterpretations, or negligent acts from relied upon data. Judgment has been used by Palmer in the interpretation of the information provided but subsurface physical and chemical characteristics may differ from regional scale geology mapping and vary between or beyond well/borehole locations given the inherent variability in geological conditions.

Palmer is not a guarantor of the geological or groundwater conditions at the subject site but warrants only that its work was undertaken, and its report prepared in a manner consistent with the level of skill and diligence normally exercised by competent geoscience professionals practicing in the Province of Ontario. Our findings, conclusions and recommendations should be evaluated in light of the limited scope of our work.

The information and opinions expressed in the Report are for the sole benefit of the Client. NO OTHER PARTY MAY USE OR RELY UPON THE REPORT OR ANY PORTION THEREOF WITHOUT PALMER'S WRITTEN CONSENT AND SUCH USE SHALL BE ON SUCH TERMS AND CONDITIONS AS PALMER MAY EXPRESSLY APPROVE. Ownership in and copyright for the contents of the Report belongs to Palmer. Any use which a third party makes of the Report is the sole responsibility of such third party. Palmer accepts no responsibility whatsoever for damages suffered by any third party resulting from use of the Report without Palmer's express written permission. Should the project design change following issuance of the Report, Palmer must be provided the opportunity to review and revise the Report in light of such alteration or variation. Memorandum Page 11 | March 26, 2024 Supplemental Hydrogeological Assessment: Short-Term Dewatering and Long-Term Seepage Estimate Updates

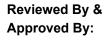


6. Signatures

Thank you for the opportunity to support The Brock Zents Partnership with this interesting project. Should there be any question on this report, please contact the undersigned.

Prepared By:

Tanvi Patel, M.EnvSc., G.I.T. Hydrogeologist-In-Training





Jason Cole, M.Sc., P.Geo. VP, Principal Hydrogeologist

References

- Armstrong, D.K., and Dodge, J.E.P., 2007: Paleozoic Geology Map of Southern Ontario. Ontario Geological Survey, Miscellaneous Release Data 219.
- Chapman, L.J., and Putnam, D.F., 2007: The Physiography of Southern Ontario. Ontario Geological Survey, Miscellaneous Release Data 228.

Ontario Geological Survey, 2003: Surficial Geology of Southern Ontario.

Terrapex Environmental Ltd., 2022: Hydrogeological Review. Reference CT2694.03

Terrapex Environmental Ltd., 2024: Supplementary Dewatering and infiltration Study. Reference CT2694.



Attachment A – Design Drawings (GMB Architecture, 2023)



BUILDING / SITE STATISTICS

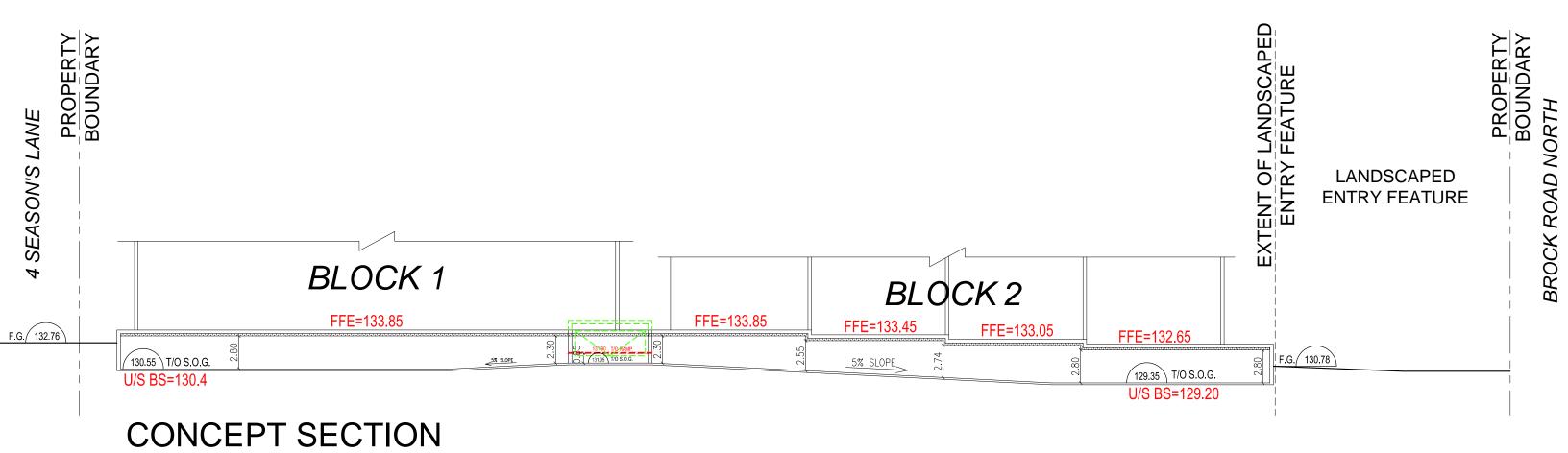
4 STOREY STACKED: 162 UNITS 3 STOREY REAR LANE STACKED TOWNS: 82 UNITS 3 STOREY REAR LANE STANDARD TOWNS: 30 UNITS TOTAL: 274 UNITS

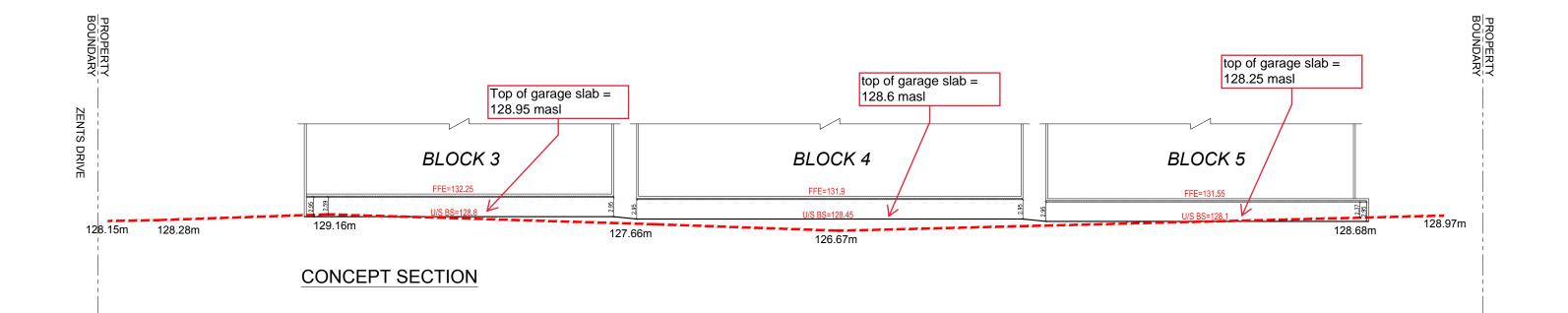
PARKING STATS

4 STOREY STACKED: 162 SPACES @ 1 SPACE / UNIT 3 STOREY REAR LANE STACKED TOWNS: 164 SPACES @ 2 SPACES / UNIT 3 STOREY REAR LANE STANDARD TOWNS: 60 SPACES @ 2 SPACES / UNIT TOTAL RESIDENT PARKING PROVIDED: 386 SPACES TOTAL VISITOR PARKING PROVIDED: 64 SPACES @ 0.23 SPACES / UNIT TOTAL PARKING PROPOSED: 446 SPACES

- Contractor shall check all dimensions and report any discrepancies to the Architect before proceeding with the
- The contractor and/or engineeer shall verify u/s footing elevations and soil bearing capacity prior to excavation an commencement of work.
- DO NOT SCALE DRAWINGS

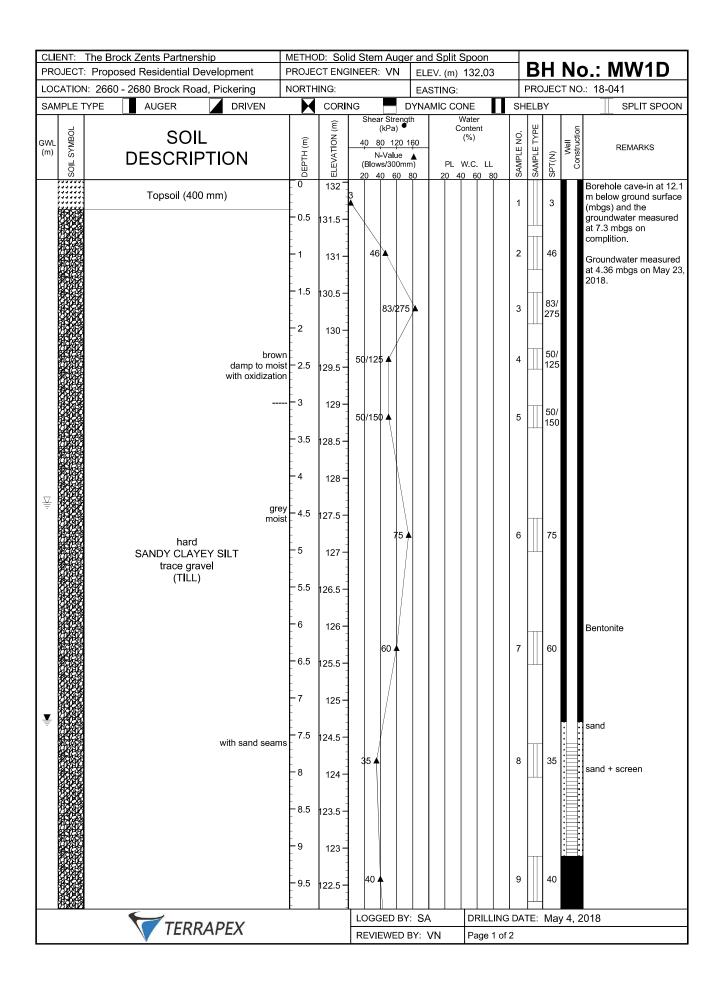








Attachment B - Borehole Logs (Terrapex, 2022 & 2024)



	he Brock Zents Partnership					Split Spoon				
	Proposed Residential Development	-		SINEER: VN	-	V. (m) 132.03				o.: MW1D
	2660 - 2680 Brock Road, Pickering	NORTH			_			PROJECT NO.: 18-041		
GWL (m)	AUGER DRIVEN	DEPTH (m)	ELEVATION (m)	Shear Stren (kPa) 40 80 120 N-Value (Blows/300r 20 40 60	oth 160 ▲ nm)	Water	SAMPLE NO. SAMPLE TYPE		Well Construction	REMARKS
	very dense, wet, grey SAND AND SILT trace gravel, trace clay (TILL) with sand seams and layers	- 10 - 10.5 - 11 - 11.5 - 12	122 - 121.5 - 121 - 120.5 - 120.5 - 120.5 -				10	50/ 150		Augering through rock/ boulder
2000 2000 2000 2000 2000 2000 2000 200	very dense, wet, grey SANDY GRAVEL	- 12.5	119.5 — 119.5 — 119 —	50/150 ▲			11	50/ 150		
	hard, damp, grey CLAYEY SILT	-	118.5	50/100 ▲			12	50/ 100		Augering through rock/ boulder
				LOGGED B	Y: SA	DRILLING E	DATE:	May	y 4, 2	2018
	TERRAPEX			REVIEWED	BY: V	N Page 2 of 2				

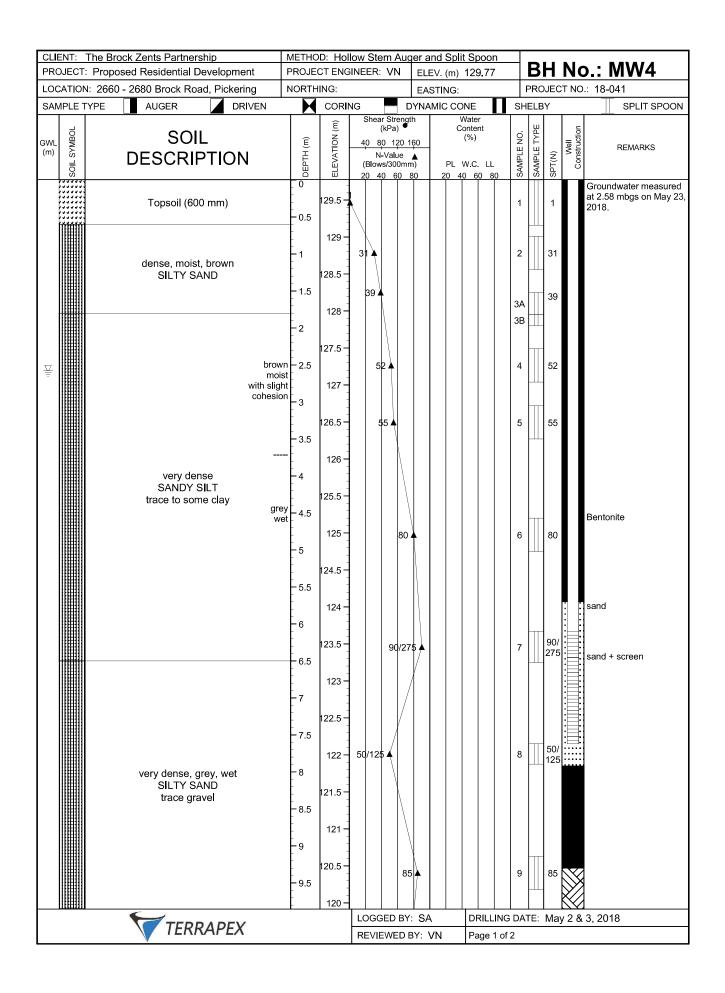
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	BOL	SOIL	Ē	ELEVATION (m)		Shea I			I		Conte (%	ent		V	SAMPLE TYPE		Well Construction	
GWL (m)	SYM	DESCRIPTION	LH (P	ATIC		10 8 N	-Valu	ie j						PLE	PLE 1	î	Wel	REMARKS
	SOIL SYMBOL	DESCINITION	DEPTH (m)	ELEV		(Blow 20 4	/s/300	Jmm)		L W.C			SAMPLE NO.	SAMI	SPT(N)	ပိ	
			<u> </u>	132			Ň	<u> </u>	ľ		Ť		Ĭ					Groundwater measured
			-	-														at 0.81 mbgs on May 23, 2018.
			- 0.5	131.5 –														
Ā			-	-														
			- 1 -	131 -														
			-	-														
			- 1.5	130.5 -														
			-	-														Bentonite
			-2	130 -														
		Straight auger to install the monitoring well	-	-														
			2.5	129.5 -													: :	Sand
			-	-													:	
			-3	129 -														1
			-	-														
			- 3.5	128.5 -													:	Sand + Screen
			-	-														
			-4	128 -														1
			-	-														
			4.5	127.5 -													:	
		END OF BOREHOLE															•	
			•		L(OGG	ED	BY:	SA	\		RILL	.ING	DAT	ΓE:	May	7,2	2018
		TERRAPEX			R	EVIE	EWE	DВ	Y: \	٧N	Р	age	1 of [·]	1				

				id Stem Auge INEER: VN		Split Spoon V. (m) 131.44		BI	Η	Nc	o.: BH2
LOCATION:	2660 - 2680 Brock Road, Pickering	NORTH	HNG:		EAS	TING:					: 18-041
SAMPLE TY	/PE AUGER DRIVEN		COR				SH	ELB	Y		SPLIT SPOON
Soll SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Strend (kPa) 40 80 120 N-Value (Blows/300m 20 40 60	160 m)	Water Content (%) PL W.C. LL 20 40 60 80	SAMPLE NO.	SAMPLE TYPE	SPT(N)	Well Construction	REMARKS
	Topsoil (300 mm)	– 0	-	24			1A				Borehole cave-in at 7.6 mbgs and the
	compact, moist, brown SANDY SILT	- 0.5	131 -				1B		24		groundwater measured at 6.7 mbgs on complition.
	hard, damp, brown SANDY CLAYEY SILT	- - - - -	130.5	60			2		60		
	trace gravel (TILL)	- 1.5	130 - - - 129.5 -	50/150 🔺			3		50/ 150		
	brown damp to mois with oxidizatior	t - 2.5	129 -	50/125 ▲			4		50/ 125		
	with oxidization	3	- 128.5 	50/100 ▲			5		50/ 100		
		- 3.5	128 - 								
	grey moist to we		- - - - - - -								
	very dense SANDY SILT trace gravel trace to some clay	- 5	126.5	52 ▲			6		52		
	(TILL) with sand seams and layers	- 5.5 - - - 6	126 - - - 125.5 -								Augering through rock boulder
		- 6.5	125 -	50/100 ▲			7		50/ 100		
		- - - 7 - -	124.5 - - -								
		- 7.5 	124 — - - 123.5 —	50/150 ▲			8		50/ 150		Augering refusal due to
E	END OF BOREHOLE	-	-								boulder
	TEDDADEV	1	1	LOGGED B	r: SA	DRILLING	DAT	ιι ΓΕ:	Мау	/ 4, 2	018
	TERRAPEX			REVIEWED	BY: V	N Page 1 of	1				

N. SOIL DESCRIPTION Source formulation (%) Water (%) Water (%) Water (%) Source (%)	The Brock Zents Partnership			id Stem Aug				۲.	No	• MW3D
AMPLE TYPE AUGER DRNEN CORING PUNANDE CONE SHELEY SPLIT SPOC 00 00 00 00 00 00 00 00 00 00 00 00 00				DINEER. VIN		130.37				
No. SOIL DESCRIPTION Sould be and be an				NG		ONE				SPLIT SPOON
Topsol (600 mm) 0 130 1 3 1 3 1 3 1 1 3 1 1 1 3 1 1 1 3 1 <th1< th=""> 1 1</th1<>			ELEVATION (m)	40 80 120 N-Value (Blows/300m	160 nm) PL	Content (%) W.C. LL	SAMPLE NO.	SPT(N)		
compact to dense, moist, brown silly sand, trace gravel, trace clay (Probable FLL) 1 2	Topsoil (600 mm)	-		3			1	3	n g	nbgs and the proundwater measured it 2.4 mbgs on
very dense, brown, moist 28.5 42 4 52 very dense, brown, moist 27.5 57 5 57 sharp Sharp -2.5 128 52 6 57 with slight cohesion intermixed with TILL layers -3.5 127 57 6 50/125 -4.5 126.5 -4.5 126.5 -5 50/125 6 50/125 -5.5 125.5 50/125 -6.5 126.5 -7 123.5 50/125 -5.5 126.5 -7 123.5 7 7 7 7 50/125 -5.5 126.5 -7 123.5 50/125 <td>silty sand, trace gravel, trace clay</td> <td>-</td> <td>-</td> <td>25</td> <td></td> <td></td> <td>2</td> <td>25</td> <td>a</td> <td>Groundwater measured at 2.67 mbgs on May 23 2018.</td>	silty sand, trace gravel, trace clay	-	-	25			2	25	a	Groundwater measured at 2.67 mbgs on May 23 2018.
Very dense. brown, moist SANDY SILT with slight cohesion intermixed with TILL layers 3 127.5 127 4 57 57 57 57 4.5 126-5 50/125 50/125 6 50/125 57 6 50/125 very dense SANDY SILT with slight cohesion intermixed with TILL layers 6 126-5 50/125 50/125 6 125.5 50/125 50/125 very dense SANDY SILT to SANDY SILT to SITEDEADEY 124-7 7.5 71/1275 7 7 71/1275 very dense SANDY SILT to SITEDEADEY 9 50/125 50/125 50/125 8 100 8 100 Very dense SANDY SILT to SITEDEADEY 100 121.5 121 50/156 9 50/125 8 100 9 50/125		-	128.5 - 	42			3	42		
very dense, brown, moist sANDY SLT with slight cohesion intermixed with TLL layers -3.5 127 57 57 -4.5 126.5 -4.5 50/128 6 50/128 -5.5 125.5 -50/128 6 50/128 -5.5 126.5 -50/128 7 127 -5.5 126 -50/128 -50/128 -50/128 -5.5 126 -50/128 -50/128 -50/128 -5.5 126 -7/1275 -7/1275 -7/1275 -5.5 123 -7/1275 -7/1275 -7/1275 -5.5 123 -7/1275 -7/1275 -7/1275 -5.7 123 -50/128 -7/1275 -7/1275 -5.6 -7/123 -7/1275 -7/1275 -7/1275 -5.5 122 -50/126 -7/1275 -7/1275 -7/123 -7/123 -50/126 -7/1275 -7/1275 -7/123 -7/123 -7/1275 -7/1275 -7/1275 -7/123 -7/123 -7/123 -7/1275 -7/1235 -7/123 -7/12		-	-	52 🔺			4	52		
-4.5 126 -4.5 126 50/125 • 50/125 • -5.5 125- -5.5 125- -5.5 125- -5.5 124- -6.5 124- -6.5 124- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -7.5 123- -8.5 122- -8.5 122- -9 121- -9.5 121- -9.5 121- -9.5 121- -9.5 121- -9.5 121- -9.5 121- -9.5<	SANDY SILT with slight cohesion	-	-	57 🔺			5	57		
very dense		-	-							Bentonite
very dense		- - - - - - -	-	50/125			6	50/ 125		
Very dense SANDY SILT to SILTY SAND trace gravel	11	^{/n} – 6	-							
SANDY SILT to SILTY SAND trace gravel		- - - - - -	-	71/275			7			
grey wet 8 122.5 8.5 122 9 121.5 9 121.5 9.5 121 50/150 9 121 50/150 9.5 121 50/150 9 121 50/150 9.5 121 50/150 9 121 50/150 9 150 121 50/150 9 150	SANDY SILT to SILTY SAND	-	123 -	50/125			8	50/		
9 121.5 9 121.5 9.5 121 50/150 9 121 50/150 9.5 121 50/150 9 150 150 121 50/150		et - 0	-							
121 50/150 9.5 121 121 10/150 100 100 100 <		-	- 121.5 – 					50/		
		- 9.5	121 - - - -					150		
REVIEWED BY: VN Page 1 of 2	TERRAPEX							Ma	y 3, 20	18

CLIENT:	The Brock Zents Partnersh	iip	METHO					-	and	Split	Spo	on							_
	Proposed Residential De		PROJEC		INE	ER:	VN	_		V. (m)		.37						<u>MW3</u>	D
	N: 2660 - 2680 Brock Road		NORTHI							TING:			_			T NO	: 18-0		
SAMPLE 1	TYPE AUGER	DRIVEN		CORIN		hear	Stre				DNE Water		SH	ELB	iY I			SPLIT SI	POON
Soll SYMBOL	SOIL DESCRIP		DEPTH (m)	ELEVATION (m)	40 (B	0 <u>80</u> N-\ Blows) 12 Va l u s/300	0 160 e ▲ 0mm) 0, 80	0	С	Conter (%) W.C.	ıt LL	SAMPLE NO.	SAMPLE TYPE	SPT(N)	V/ell Construction		REMARKS	
	very dense, we SILTY SAN	t, grey ID	- 10.5 - 10.5 - 11 - 11.5 - 12.5 - 12.5 - 13.5 - 13.5	120.5 120 119.5 119 118.5 118 118 117.5 117 117		125		6/25	0				10		96/ 250 50/ 125 50/ 125				
	END OF BOREHOLE																		
	TERR	APEX			<u> </u>			BY: D BY		'N	_	ge 2		ſE:	Ma	/ 3, 2	018		

		The Brock Zents Partnership	METHC PROJE						Split V. (m)			_	B	н	No	o.: MW3S
			NORTH						TING:							0.: 18-041
	MPLE		Π	COR	NG		D		AIC CO				ELE			SPLIT SPOON
GW (m	SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	40 (Bl	near Str (kPa 80 1: N-Valu lows/30 40 6	20 10 ue , 0mm	<u>60</u> ▲ 1)	PL	Water Conten (%) W.C.	t LL	SAMPLE NO.	SAMPLE TYPE	SPT(N)	Well Construction	REMARKS
Ţ		Straight auger to install the monitoring wel	- 0.5	129.5 129.5 129 128.5 128 128 128 128 128 128 128 128 128 128												Groundwater measured at 1.62 mbgs on May 23, 2018. Sand Sand + Screen Bentonite
	1	END OF BOREHOLE														
F		TERRAPEX	-		-	GGED			'N	-	ILLING		TE:	Ma	7,2	2018



CLIENT:	The Brock Zents Partnership	METHO	D: Hol	llow Stem A	uger a	and Split Spoon			
PROJECT:	Proposed Residential Development	PROJE	CT ENG	GINEER: VN	ELE	EV. (m) 129.77	B	<u>H No</u>	o.: MW4
	I: 2660 - 2680 Brock Road, Pickering	NORTH				STING:			.: 18-041
SAMPLE T	YPE AUGER DRIVEN		COR				SHELE	3Y	SPLIT SPOON
GWL (m)	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Stre (kPa) 40 80 12 N-Valu (Blows/300 20 40 60	0 160 ∋ ▲ mm)	Water Content (%) PL W.C. LL 20 40 60 80	SAMPLE NO. SAMPLE TYPE	SPT(N) Well	REMARKS
	very dense, wet, grey SAND trace to some gravel trace silt	- 10.5 - 10.5 - 11 - 11.5 - 12.5 - 12.5 - 13	119.5 - 	71 50/125 ▲			10	71	Augering through rock/ boulder
	hard, grey, moist CLAYEY SILT END OF BOREHOLE	- 13.5 - 14	116.5 - - - - - - - - - - - - - - - - - - -	50/150 ▲			12	50/	
	TERRAPEX		I	LOGGED	3Y: SA	A DRILLING	DATE:	May 2 &	3, 2018
	V IERRAPEA			REVIEWE	DBY:	VN Page 2 of 2	2		

CLIENT: The Brock Zents Partnership PROJECT: Proposed Residential Development			low Stem Aug SINEER: VN			R	н	Nc	o.: MW5
LOCATION: 2660 - 2680 Brock Road, Pickering	NORTH			ELEV. (m) EASTING:	131.39				: 18-041
SAMPLE TYPE AUGER DRIVEN		CORI		DYNAMIC CC		HELE			SPLIT SPOON
	DEPTH (m)	ELEVATION (m)	Shear Streng (kPa) • 40 80 120 · N-Value (Blows/300m 20 40 60	th C 160 m) PL	Water	SAMPLE NU.		Well Construction	REMARKS
black, moist, sand and gravel (FILL)	0.5	131.5 - - - 				1	8		Groundwater measured at 5.56 mbgs on May 23, 2018.
	- - - - - - - - - - - - - - - - - - -	130.5 - - 130 -	30			2	30		
brov brov dar		129.5 -	50/150			4	66 50/ 150		
		128.5	72			5	72		
gr gr gr mo	ey	128 - 							
very dense SAND and SILT trace gravel, trace clay (TILL) ⊊	- - - - - - - - - - - - - - - - - - -	126.5 -	52			3	52		
	- - - - - - - - - - - - - - - - - - -	125.5 — - - 125 —	46 ▲			7	46		Bentonite Augering through rock/ boulder
	- 7 - 7 - 7 - 7.5	124.5 - - - 124 -	5Q/150 🔺			з 💷	50/	:=:1	sand
	- 8	123.5 - - - - 123 -					150		sand + screen Augering through rock/ boulde
END OF BOREHOLE	- 9	122.5 -	50/125 🔺			э 🎞	50/ 125		
TERRAPEX			LOGGED BY REVIEWED		DRILLING D Page 1 of 1	ATE:	 May	y 1, 2	018

DCATION: 2660 - 2680 Brock Road, Pickering AUGER DRIVEN AMPLE TYPE AUGER DRIVEN W U U U U U U U U U U U U	(E) HLddd 0 0.5 1 1.5 2 2.5 12 3 3.5 12	NG: CORIN (m) NOILENALION		hear (k 80 N-V	Strer (Pa) 12(Value (/300r 0 60	EA DYN, ngth 0 160	- PL	:		F		JEC		D.: BH6 D.: 18-041 SPLIT SPOON REMARKS Borehole cave-in at 11.3 mbgs and the groundwater measured at 0.6 mbgs on complition.
AMPLE TYPE AUGER DRIVEN	(iii) HLd and O 0.5 13 1 1.5 12 2 2.5 12 3 3.5 12	CORIN (III) NOLLEYATI 30.5 - 130 - 129 - 129 - 128 - 128 - 128 - 128 -	SI 40 (B 20 8 8 4	80 N-V 40 17 51	12C	DYN, ngth 0 160	AMIC C	Water Content (%)		c SAMPLE NO.	SAMPLE TYPE	(N)LdS 8 17 40	Well Construction	REMARKS Borehole cave-in at 11.3 mbgs and the groundwater measured at 0.6 mbgs on
Topsoil (600 mm)	$\begin{array}{c cccc} 0 & & \\ 0.5 & 1^{3} \\ 1 & & \\ 1.5 & 1^{2} \\ 2 & & \\ 2.5 & 1^{2} \\ 3 & \\ 3.5 & 1^{2} \end{array}$	30.5 - 130 - 29.5 - 129 - 28.5 - 128 - 27.5 -	40 (B 20 8 8 4	80 N-V 40 17 51	12C) 160 	- PL	Content (%) W.C.		1		8 17 40	Well Construction	Borehole cave-in at 11.3 mbgs and the groundwater measured at 0.6 mbgs on
Topsoil (600 mm)	$\begin{array}{c cccc} 0 & & \\ 0.5 & 1^{3} \\ 1 & & \\ 1.5 & 1^{2} \\ 2 & & \\ 2.5 & 1^{2} \\ 3 & \\ 3.5 & 1^{2} \end{array}$	30.5 - 130 - 29.5 - 129 - 28.5 - 128 - 27.5 -	8	17 -0 51						1		8 17 40		mbgs and the groundwater measured at 0.6 mbgs on
to dense	1.5 ¹² 2 2.5 ¹² 3 3.5 ¹²	29.5 - 129 - 28.5 - 128 - 27.5 -	4	51						3		40		
grey moist to wet very dense SAND AND SILT trace gravel, trace clay (TILL) with sand layers and seams	2 2.5 3 3.5	129 - 28.5 - 128 - 27.5 -		51										
moist grey moist to wet very dense SAND AND SILT trace gravel, trace clay (TILL) with sand layers and seams	3 3.5 ¹²	128 - 	50/							4		51		
grey moist to wet very dense SAND AND SILT trace gravel, trace clay (TILL) with sand layers and seams	3.5 ¹²	27.5	50/	125										
moist to wet very dense SAND AND SILT trace gravel, trace clay (TILL) with sand layers and seams	4	127 -			- IV					5		50/ 125		
Trace gravel, trace clay (TILL)	1	-		75	5/275	5				6		75/ 275		
	4.5	26.5 - 126 -			70					7		70		
	5.5 ¹²	25.5 -												
	0	125 – 24.5 –	50/	125						8		50/ 125		
	7	124 -												
	7.5	23.5	50/	125						9		50/ 125		
		22.5												
GRAVELLY SAND	5	122 - 			91/	275				10		91/ 275		
TERRAPEX		-	LO	GGE	ED E) BY: S,	 A	DRIL	LING	DAT	 FE:	Apri	il 30,	, 2018

PROJECT HONDELER: VN ELEV (m) 130.94 BH LOCA 18.041 LOCATION: 2860-2680 Brock Road, Pickering DRIVEN EASTING: PROJECT NO: 18.041 SAMEL TYPE June T Market Market DRIVEN COMING With Second Road, Pickering SALT SPOD DM SOLL BESCRIPTION E E SALT SPOD Second Road, Pickering Second Pickering Second Pickering	CLIENT: T	The Brock Zents Partnership	METHO	D: Sol	id Ste	em Ai	uger	r and	d Split S	Spoo	n						
SAMPLE TYPE AUGER DRIVEN CORING DUMADIC COVE SAULEY SPLIT SPOO 001 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3					GINEE	R: VI	۷	ELE	EV. (m)	130.	94						
SOL (m) Sol (m) <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>TNO</td><td></td><td></td></t<>															TNO		
very dense, wet, grey trace gravel, trace clay (TLL) with sand seams and layers 10.5 20.5 11.1 10.5 11 12 119 12.5 11.5 50/272 12 12 very dense, wet, grey with sand damp, grey SAND AND SLT trace gravel, trace clay (TLL) 12.5 11.5 50/272 12 12 50/ 275 Nerd damp, grey SAND AND SLT trace gravel, trace clay (TLL) 13.6 17.5 50/272 13.7 60/ 10.5 17.5 END OF BOREHOLE ID GRAVEL 10.5 17.5 17.5 17.5 10.5 10.5	GWL (m)	SOIL			Sho 40 (Blo	(kPa 80 1 N-Val ows/30	rength 20 10 ue 4 00mm	h 60 ▲ 1)	V Ca PL \	Vater ontent (%) W.C.	LL				Vvell Construction		
very dense, wet, grey 12.5 18.5 50275 + 12 12 50/275 13 118- 13 118- 13 118- 13 118- Wery dense, wet, grey 13 118- 13 118- 13 118- 13 118- Wery dense, wet, grey 13 118- 13 118- 13 118- 13 118- 13 118- 13		SAND AND SILT trace gravel, trace clay (TILL)	- 10.5	120.5 								11					
trace gravel, trace clay 13.3 (Possible BEDROCK) with shale pieces 50/20 13.7 50/20 END OF BOREHOLE 10.1 10.1 10.1 10.1 Image: state s		SANDY GRAVEL	- 12.5 - 13		50/2	75▲						12					
		(TILL) with shale pieces			50/	20						<u>13</u>		50/		(Possible BEDROCK	.)
LOGGED BY: SA DRILLING DATE: April 30, 2018																	
TERRAPEX REVIEWED BY: VN Page 2 of 2		TERRAPEX		1	-								I TE:	ı Apr	il 30,	2018	

		The Brock Zents Partnership Proposed Residential Development			low Stem Au BINEER: VN		and Split S EV. (m) 12			Bł	-	No	.: BH7
LOC	ATION	: 2660 - 2680 Brock Road, Pickering	NORTH	ING:		EA	STING:						18-041
SAM	IPLE T	YPE AUGER DRIVEN		COR					SH	ELB	(SPLIT SPOON
WL (m)	SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Strenc (kPa) 40 80 120 N-Value (Blows/300m 20 40 60	160 (m)	Wa Coni (% PL W. 20 40	c. LL	SAMPLE NO.	SAMPLE TYPE	SPT(N)	Well Construction	REMARKS
		Topsoil (150 mm)	0	-	7			ÎÎ					Borehole cave-in at 4.5
		firm, dark brown , moist clayey silt (FILL) with sandy gravell layer	- 0.5	129.5 - 129 - 129 -	▲ 6		23 18		1 2		7 6		nbgs and the groundwater measured at 1.8 mbgs on complition.
		compact moist, brown	-2	128 -	20		15 •		3		20	s	The borehole moved 3 south due to an existing concrete foundation
- 10 - 10 - 10 - 10 - 10 - 10 - 10 - 10		SANDY SILT trace gravel, trace clay (TILL) with oxidization very dense	- 2.5 - - - 3	127.5 -	26		10		4		26		
			- 3.5	126.5 -	55 ▲		8		5		55		
		brow moi with clayey silt seam	st 4.5	126 - 125.5 - 125 - 125 -	50/250 ▲		12		6		50/ 250		
		very dense, wet SILTY SAND	- 5.5 - 6 - 6.5 7	124 - 123.5 - 123 -	34		19		7		34		
		gre moist to w	- 7.5 et	122.5 - 122 - 122 -	50/125 ▲		18		8		50/ 125		
			- 9	121 – 120.5 –	50/125 ▲		16		9		50/ 125		
		TEDDADEV	•		LOGGED BY	Y: SA	<u> </u>	RILLING	DAT	ΓE: /	April	30, 2	2018
		TERRAPEX			REVIEWED	BY: '	VN F	Page 1 of	2				

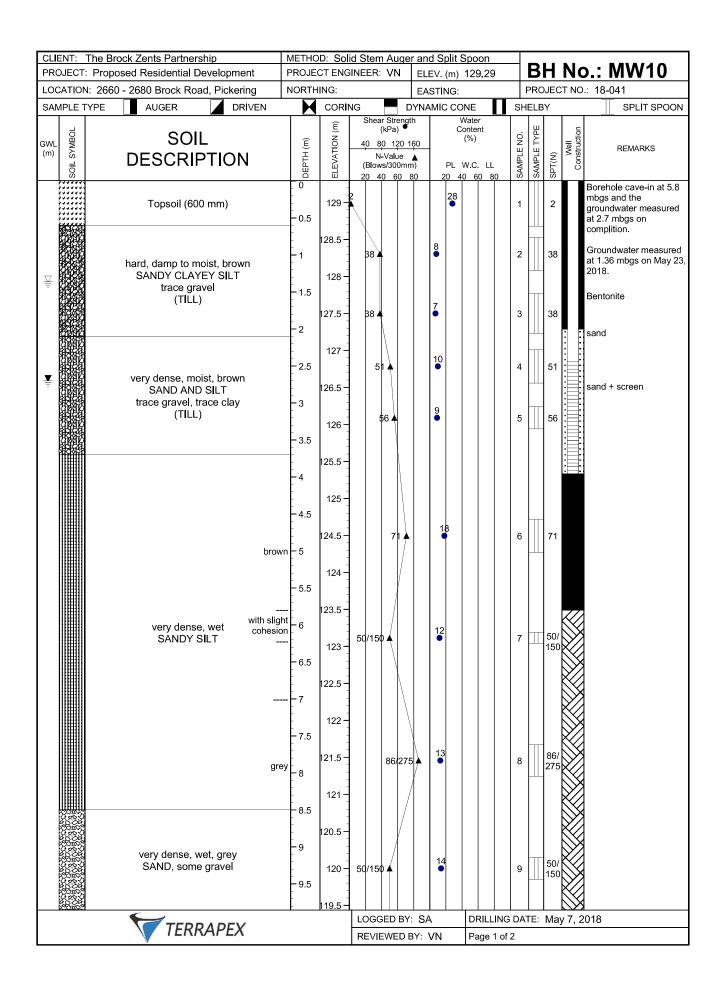
	The Brock Zents Partners		METHC												NL		
	Proposed Residential D		PROJE		INEE	R: VI	N		EV. (m)		91					<u>o.: BH7</u>	
	N: 2660 - 2680 Brock Roa		NORTH						STING:						I NO	.: 18-041	0.011
SAMPLE 1	TYPE AUGER	DRIVEN		CORI		ear Str			MIC CO	ONE Water		SH T	ELE T				OON
Soll Symbol (m)	SOII DESCRIF		DEPTH (m)	ELEVATION (m)	40 (Bl	ear Str (kPa 80 1 N-Val ows/30 40 6	20 1) ue - 10mm	60 ▲ 1)	PL	Content (%) W.C.		SAMPLE NO.	SAMPLE TYPE	SPT(N)	Vell Construction	REMARKS	
	very dense, w SILTY SA	/et, grey ND	- 10 - 10.5 - 11 - 11.5 - 12 - 12.5 - 13.5 - 13.5 - 14			25	78		13 16 17			10		50/ 125 78 68			
					LOO	GGED	BY:	: SA				→ → DA*	TE:	Apr	il 30,	2018	
	TERR	RAPEX			-	/IEWE				_	e 2 of						

	The Brock Zents Partnership			id Stem Aug BINEER: VN					R	н	N	o.: MW8D
	Proposed Residential Development : 2660 - 2680 Brock Road, Pickering	NORTH		DINEER. VIN		EV. (m) STING:	131.64					D.: 18-041
SAMPLE T		M	COR	NG	_		NE		ELB			SPLIT SPOON
Solt SYMBOL	SOIL DESCRIPTION	O DEPTH (m)	ELEVATION (m)	Shear Stren (kPa) 40 80 120 N-Value (Blows/300n 20 40 60	160 ▲ nm)	PL V	Vater ontent (%) V.C. LL) 60 80	SAMPLE NO.	SAMPLE TYPE	SPT(N)	Well Construction	REMARKS
	Topsoil (600 mm)	- 0.5	131.5 - 131 -	3		30 •		1		3		mbgs and the groundwater measured at 2.7 mbgs on complition.
		- - - - - -	130.5 -	13		11		2		13		Groundwater measured at 5.11 mbgs on May 23, 2018.
	hard, damp to moist, brown	- 1.5 - 2	130 -	39		9		3		39		
- I I	SANDY CLAYEY SILT trace gravel (TILL)	- 2.5	129.5 – - 129 –	67		10		4		67		
		- 3 - 3.5	128.5 - - - - - - - - - - - - - - - - - - -	٤	34	9		5		84		
	very dense, moist to wet, grey SAND AND SILT trace gravel, trace clay (TILL)	4.5 - 5.5 - 6.5 - 7	127.5 - 127 - 126.5 - 126 - 125.5 -	50/150 ▲ 73		8		6		50/ 150 73		
	very dense, wet, grey SILTY SAND trace gravel	- - - - - - - - - - - - - - - - - - -	124.5 - 124 - 123.5 - 123.5 -	50/125 ▲		8		8		50/ 125	• •	Bentonite sand
		- 9 - 9.5	122.5 -	50/75 ▲		12		9		50/ 75		sand + series
	TEDDADEV	<u> </u>	<u> </u>	LOGGED B	Y: SA		DRILLING	G DA	TE:	May	u⊟. / 2, 2	sand + screen 2018
	TERRAPEX			REVIEWED	BY:	VN	Page 1 o	f 2				

CLIENT:	The Brock Zents Partnership	METHO	D: Sol	id Stem Aug	er an	id Split Spoor	1				
	Proposed Residential Development	1		GINEER: VN		EV. (m) 131.6					: MW8D
	: 2660 - 2680 Brock Road, Pickering	NORTH			_	STING:				T NO.: '	
SAMPLE T	YPE AUGER DRIVEN		COR			AMIC CONE Water		SHEL	.BY	<u> </u>	SPLIT SPOON
GWL SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Strer (kPa) 40 80 120 N-Value (Blows/300r 20 40 60	160 ▲ nm)	PL W.C. 20 40 60	_L 80	SAMPLE NO.	SPT(N)	Viell Construction	REMARKS
មានទំនាំកំណើន និងកំណើន និងកំណើ និងកំណើន និងកំណើន និង និងកំណើន និងកំណើន និង	very dense, wet, grey GRAVELLY SAND	- 10 - 10.5 - 11 - 11.5 - 12	121.5 	50/100 ▲		6		10	50/ 100	Se	nd + Screen
	very dense, wet, grey SAND AND SILT trace gravel, trace clay	- 12.5 - 13.5	119.5	50/150 ▲		11		11]	50/	AL	gering through rock/ ulder
	(TILL) with occasional sand semas and layers	- 14 - 14.5 - 14.5	117.5 	50/275 ▲		13		12	50/ 275		DSSIBLE BEDROCK
	END OF BOREHOLE								124		
	TERRAPEX			LOGGED B REVIEWED			LING I		: Ma	y 2, 201	8

		The Brock Zents Partnership			D: Sol								B	BH	N	o.: MW8S
		1: 2660-2680 Brock Road, F		NORTH		GINEER: VN ELEV. (m) 131.033 EASTING:			PROJECT NO.: CA18-041							
SAMPLE TYPE AUGER DRIVEN										SHELBY SPLIT SPOON						
GWL (m)	SOIL SYMBOL	SOIL DESCRIPT	_	DEPTH (m)	ELEVATION (m)	Sh 40 (Bl	ear Str (kPa 80 12 N-Valu ows/30	ength) 20 16 Je Omm	1 50	V Ca PL N	Vater ontent (%) N.C. LL		SAMPLE NO. SAMPLE TYPE	SPT(N)	Well Construction	
		Straight auger to : hard, damp CLAYEY SANDY trace grave (TILL) END OF BOREHOLE	2.28 m , brown ⁄ SILT	- 0.5 - 1.5 - 1.5 - 2.5 - 3.5 - 3.5	131 - 130.5 - 130.5 - 129.5 - 128.5 - 128.5 - 127.5 - 127 - 127 -		50/12 7 7							50/ 125 72 71		Borehole open and dry on completion. Groundwater was measured at 2.72 m on June 26, 2019. Bentonite Sand Sand and Screen
	TERRAPEX						LOGGED BY: LG DRILLING DATE: June 12, 2019 REVIEWED BY: VN Page 1 of 1				, 2019					

	he Brock Zents Partnership Proposed Residential Development			INEER: VN		d Split Spoon EV. (m) 130.22	_	Bł	H N	lo.: BH9		
	2660 - 2680 Brock Road, Pickering					EASTING:			PROJECT NO.: 18-041			
SAMPLE TY			COR	NG		MIC CONE	SH	ELBY	,	SPLIT SPOON		
Soll Symbol	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Stren (kPa) 4 40 80 120 N-Value (Blows/300n 20 40 60	160 ▲ nm)	Water Content (%) PL W.C. LL 20 40 60 80	SAMPLE NO.	SAMPLE TYPE	SPT(N) Well	REMARKS		
	Topsoil (300 mm) soft, dark brown, moist	0	130 -				1		3	Borehole cave-in at 3.3 mbgs and the groundwater measured		
	clayey silt, trace gravel trace organics (FILL)	- 0.5	129.5 -	19			2A		19	at 1.5 mbgs on complition.		
¥ Solution	hard, damp, brown SANDY CLAYEY SILT trace gravel	- ' - - 1.5	129 -				2B					
	(TILL) with sand seams and layers	- 2	128.5	32			3		32			
	very dense, wet, brown	- 2.5	128 – - 127.5 –	46			4		46			
	SILTY SAND with occasional clay layers	- 3 - 3.5	127 -	52 🔺			5		52			
	hard, moist, grey SANDY CLAYEY SILT trace gravel (TILL) with wet sand seams and layers	-4	126.5	47			6 7		64 47			
			124.5 - 124 - 124 - 123.5 -	52 🔺			8		52			
	very dense, moist to wet, grey SAND AND SILT trace gravel, trace clay (TILL)	- - 7.5 - - - - - - - - - - - - - - - - - - -	123 – 122.5 – 122 – 122 – 122 – 121.5 –	69 4			9		69			
		- 9 - - 9.5	- - - - - - - -	78	3 🔺		10		78			
E							_					
	TERRAPEX			LOGGED B	Y: SA	A DRILLING	i DA1	re: A	April 3	80, 2018		



CLIENT:	The Brock Zents Partnership	METHO	D: Sol	id Stem Auge	er and	d Split Spoon						
-	Proposed Residential Development			INEER: VN	-	EV (m) 129.29	E	BH N	lo.: MW10			
	1: 2660 - 2680 Brock Road, Pickering	NORTH			_	STING:		PROJECT NO.: 18-041				
SAMPLE T	YPE AUGER DRIVEN		CORI				SHEL	BY	SPLIT SPOON			
GWL (m) GWL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Streng (kPa) 40 80 120 N-Value (Blows/300m 20 40 60	160 ▲ m)	Water Content (%) PL W.C. LL 20 40 60 80	SAMPLE NO. SAMPI F TYPF	SPT(N)	REMARKS			
	very dense, wet, grey SANDY SILT	- 10 - 10.5 - 11 - 11.5 - 12 - 12	119 - 	50/150 ▲		9	10	50/ 150				
	very dense, wet, grey SAND AND SILT trace gravel, trace clay (TILL) with sand layers END OF BOREHOLE	- 12.5 - 13 - 13.5	116.5 - - - - - - - - - - - - - - - - - - -	50/150 ▲		11	12	50/				
	TERRAPEX			LOGGED BY				⊥	2018			
				REVIEWED	BY: \	VN Page 2 of 2						

	The Brock Zents Partnership Proposed Residential Development				ng and Split Spoon	F	RH	No	.: MW101
	 Proposed Residential Development 3: 2660-2680 Brock Road, Pickering, ON 	_			ELEV. (m) 131.238 EASTING:	_			CA18-041
SAMPLE T	-	M	CORI			SHEL			
(a) Soll Symbol	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Strength (kPa) ● 40 80 120 160 N-Value ▲ (Blows/300mm) 20 40 60 80	PL W.C. LL	SAMPLE NO.	SPT(N)	Well Construction	REMARKS
	Topsoil (250 mm)	0	131 -			1A 1B	32		Borehole open and groundwater measured at 1.83 mbgs on completion.
		- - - - - - - - - - - - - - - - - - -	130.5 - - - 130 -	54 🛦		2	54		Groundwater was measured at 0.5 m on June 26, 2019.
	br	own 2	129.5 -	82/150		3	82/ 150		Bentonite
			129 - - - 128.5 -	81/150 ▲		4	81/ 150		Sand
			128 -	66		5	66		Sand and Screen
	hard, damp to moist CLAYEY SANDY SILT trace gravel	grey 4	127.5 -	70 🔺		6	70		
	(TIĽL)	- 4.5 - - 5 - - - 5.5	126.5 - - - 126 -	71		7	71		
		- 5.5 	125.5 - 125 -	48		8	48		
		- - - - - - -	124.5 – 124 – 124 –						
		- 7 <u>.5</u> - 8	123.5 - 123-5 - 123 -	55 🔺		9	55		
28675920	very dense, moist, grey SAND, trace silt	- 8.5	122.5 -	78/125		10	78/		
	END OF BOREHOLE								
	TERRAPEX			LOGGED BY: L	LG DRILLING	DATE	: Jun	e 12,	2019

	The Brock Zents Partnership			id Stem Aug BINEER: VN		and Split Spoon EV. (m) 130.695	-	В	ΗN	o.: MW102D
		NORTH				STING:	Т _Р	ROJE	ECT NO	D.: CA18-041
SAMPLE -	TYPE AUGER DRIVEN	Η	COR	NG	DYNA	AMIC CONE	SHE	LBY		SPLIT SPOON
GWL SVMBOL G(m)	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	Shear Stren (kPa) 40 80 120 N-Value (Blows/300m 20 40 60	160 (m)	Water Content (%) PL W.C. LL 20 40 60 80	SAMPLE NO.	SAMPLE TYPE	Vell Construction	REMARKS
	Topsoil (230 mm)	0.5	130.5 -	18 18			1A 1B	1	8	Borehole open and groundwater measured at 5.7 mbgs oncompletion.
	compact	- - - - - -	130 - - - 129.5 -	22			2	2	2	Groundwater was measured at 2.94 m on June 26, 2019.
	 moist to wet, brown	- 1.5	129 -	41			3	4	1	
	dense SANDY SILT	- 2	128.5 -	47			4	4	7	
Ţ		-3	127.5 -	41			5	4	1	Bentonite
	browi hard, moist gre	- 3.5 - 4 - 4 - 4 - 4.5	127 - 126.5 -	69			6	6	9	
	CLAYEY SANDY SILT	-5	126 - 125.5 -	50/150			7		0/ 50	
		- 6.5	125 - 124.5 -	3	35		8	8	5	Sand
200 200 200 200 200 200 200 200 200 200	very dense, wet, grey GRAVELLY SAND	- - - - - - - - - - - - - - - - - - -	124 – 123.5 –							Sand and Screen
0.525 0.555 0.555		- 8	123 - 122.5 -	74			9	7	4	
	hard, moist, grey CLAYEY SANDY SILT, trace gravel (TILL)	8.5	-	50/150			10A 10B	5	0/	
	END OF BOREHOLE									
	TERRAPEX	1	1	LOGGED B				E: Ji	une 12	l 2, 2019

PROJECT: Proposed Residential Development PROJECT ENGINEER: VN ELEV. (m) 130.683 BH NO.: WVV102
LOCATION: 2660-2680 Brock Road, Pickering, ON NORTHING: EASTING: PROJECT NO.: CA18-041
SAMPLE TYPE AUGER AUGER CRIVEN CORING OVER SHELBY SPLIT
GWL (m) No SOIL (m) (m) SOIL (m) (m) (m)
Image: Constraint auger to 3.66 m to install the monitoring well 10
LOGGED BY: LG DRILLING DATE: June 12, 2019 REVIEWED BY: VN Page 1 of 1

	T: Patheon Developers(Ontario) Inc.				PRO	DJECT	NO.: CT20	694.	03			R		RD OF:
	ESS: 2660-2680 Brock Rd, Pickering O			DTUNIC	4000	000.00								201
	PROVINCE: 2660-2680 Brock Rd, Picke RACTOR: Pontil		ч ∣мо	RTHING (m)			em Auger			· /	65362	.0.84	ELEV.	(m) 129.65
			(Sp	nt Sp	oon			
	HOLE DIAMETER (cm): 16.51 WELL DIA				EN SLO									
				CORING SHEAR STRI (kPa)	ENGTH	V	(NAMIC C) /ATER /NTENT (%)			(HELB	e)		T SPOON
GWL (m) SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	40 80 12 + N-VALE (Blows/300 20 40 60)mm)		W.C. LL 0 60 80	SAMPLE NO.	SAMPLE TYPE	RECOVERY (%)	SV/TOV (ppm or %LEL)	LABORATORY TESTING	WELL INSTALLATION	REMARKS
	FILL moist, brown clayey silt, trace sand trace rootlets stiff to hard moist, brown CLAYEY SANDY SILT trace gravel (TILL) very dense, wet, grey SILTY SAND very dense, wet, grey SANDY SILT	-0.5 -1 -1.5 -2.5 -3 -3.5 -4 -4.5 -5.5 -6		5 19 13 35 44 67	5	4		1 2A 2B 3 4 5 6 7 8		37 < 100 100 100 100 100 100 100	55p/0p 55p/0p 55p/0p 55p/0p 55p/0p 55p/0p 55p/0p 55p/0p	PAHs, M&I, PHCs, VOCs	A	
	END OF BOREHOLE	- 6.5	123.5 - 123 _		85			9		100<	5p/0p			
	TERRAPEX	1	I			GED BY T BY: I						DATE: 0 NG DATE	. 04-Oct-2021 ATE:	
	¥				REVI	EWED	BY: VN			PAGE	E 1 OF	1		

CLIEN	T: Patheon Developers(Ontario) Inc.				PRO	DJECT	NO.: C⁻	T2694	4.03			R		RD OF:
ADDR	ESS: 2660-2680 Brock Rd, Pickering O	N											BH	202
	PROVINCE: 2660-2680 Brock Rd, Picker	ring ON	NC	DRTHING (m				EAS	ring	(m):			ELEV.	(m)
	RACTOR: Pontil			METH										
	HOLE DIAMETER (cm): WELL DIA		· /		EEN SLO			TYPE					LANT T	
SAMP	LE TYPE AUGER DRIV	EN I I		CORINO SHEAR STR	ENGTH	,	YNAMIC WATER	CON	E		SHELB			T SPOON
SOIL SYMBOL	SOIL DESCRIPTION	O DEPTH (m)	ELEVATION (m)	(kPa 40 80 12 N-VAL1 (Blows/30 20 40 6	20_160 9E 0mm)▲	PL	ONTENT (%) W.C. LL <u>40 60 8</u>	-	SAMPLE NO. SAMPLE TYPE	RECOVERY (%)	SV/TOV (ppm or %LEL)	LABORATORY TESTING	WELL	REMARKS
	FILL moist, brown clayey silt, trace sand, trace rootlets layer of crushed limestone FILL moist, light brown silty sand, trace gravel layer of crushed limestone hard, moist, brown CLAYEY SANDY SILT trace gravel (TILL)	- 0.5		12 43 42 34 64 44	*			33	1 3A 3B 4 6	66 100 100 100	<5p/0p <5p/0p <5p/1p <5p/0p <5p/0p	M&I, PAHs BTEX F1-F4		
▼]:	dense to very dense wet, brown SAND very dense, wet, grey SANDY SILT	- 4.5 - 5.5 - 5.5		42	94/6" 🌢				7	-	<5p/0p <5p/0p	PAHs		
		6.5			90/6"				9	100	<5p/0p			
	END OF BOREHOLE													
														2024
	TERRADEV					GED B			_			DATE: 0		2021
	TERRAPEX					T BY:			-			NG DATE	:	
					REV	EWED	BY: VN			PAC	GE 1 OF	1		

CLIEN	NT: Patheon Developers(Ontario) Inc.				PRC		10.: CT2	2694	.03			R		RD OF:
	RESS: 2660-2680 Brock Rd, Pickering Ol													V203
CITY/	PROVINCE: 2660-2680 Brock Rd, Picker	ing ON	N NO	RTHING (m)): 4860	130.02	E	EAST	ING ((m):	65358	4.45	ELEV	.(m) 131.61
	RACTOR: Pontil						m Auge							
BORE	HOLE DIAMETER (cm): 12.7 WELL DIA	METER	R (cm):	5.08 SCRE	EEN SLO	DT #: 1) SAND T	TYPE:	Silio	:a #2	2	SEA	LANT	TYPE: bentonite
SAMF	PLE TYPE AUGER DRIV	EN					NAMIC (CONE	=		HELB		_ SPL	T SPOON
GWL (m) SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	SHEAR STR (kPa) 40 80 12 + N-VALt (Blows/300 20 40 60	● 20_160 JE • 20mm)	CC PL	ATER NTENT (%) W.C. LL 0 60 80		SAMPLE TYPE		SV/TOV (ppm or %LEL)	LABORATORY TESTING	WELL INSTALLATION	REMARKS
8	TOPSOIL 70mm	0	131.5 -	13										Developing to at
	SAND AND GRAVEL 100mm FILL moist, brown clayey sandy silt, trace gravel very stiff to hard		131 -	13				1	вШ	100	<5p/1p <5p/1p	PAHs M&I		Borehole dry at completion
	moist, brown CLAYEY SANDY SILT trace gravel (TILL)	- 1.5	130 <u>5</u> –	▲ 16 35 ▲							<5p/1p <5p/0p			
		- 2	129.5 -	36							<5p/0p	BTEX,		
		-3	129 -	39							<5p/0p	PHCs		
	dense to very dense moist, brown SILTY SAND	- 3.5 - - - 4	128 -					6.			<5p/0p <5p/1p			
	very dense to dense moist, grey SANDY SILT trace clay, trace gravel (TILL)	- 4.5	127.5 -		7/6"						<5p/1p	PAHs, PHCs,		
	()122/	- 5 - - - 5.5	126.5 -	50 ▲							<5p/0p	VOCs, pH		
			126 - 125.5 -	46				8	3 	100	<5p/1p			
		- 6.5		48 🛦				9)	100	<5p/1p			
	END OF BOREHOLE		125 -											
	TERRAPEX			GED BY T BY: N						DATE: 0				
	7				REVI	EWED	BY: VN			PAG	E 1 OF	1		

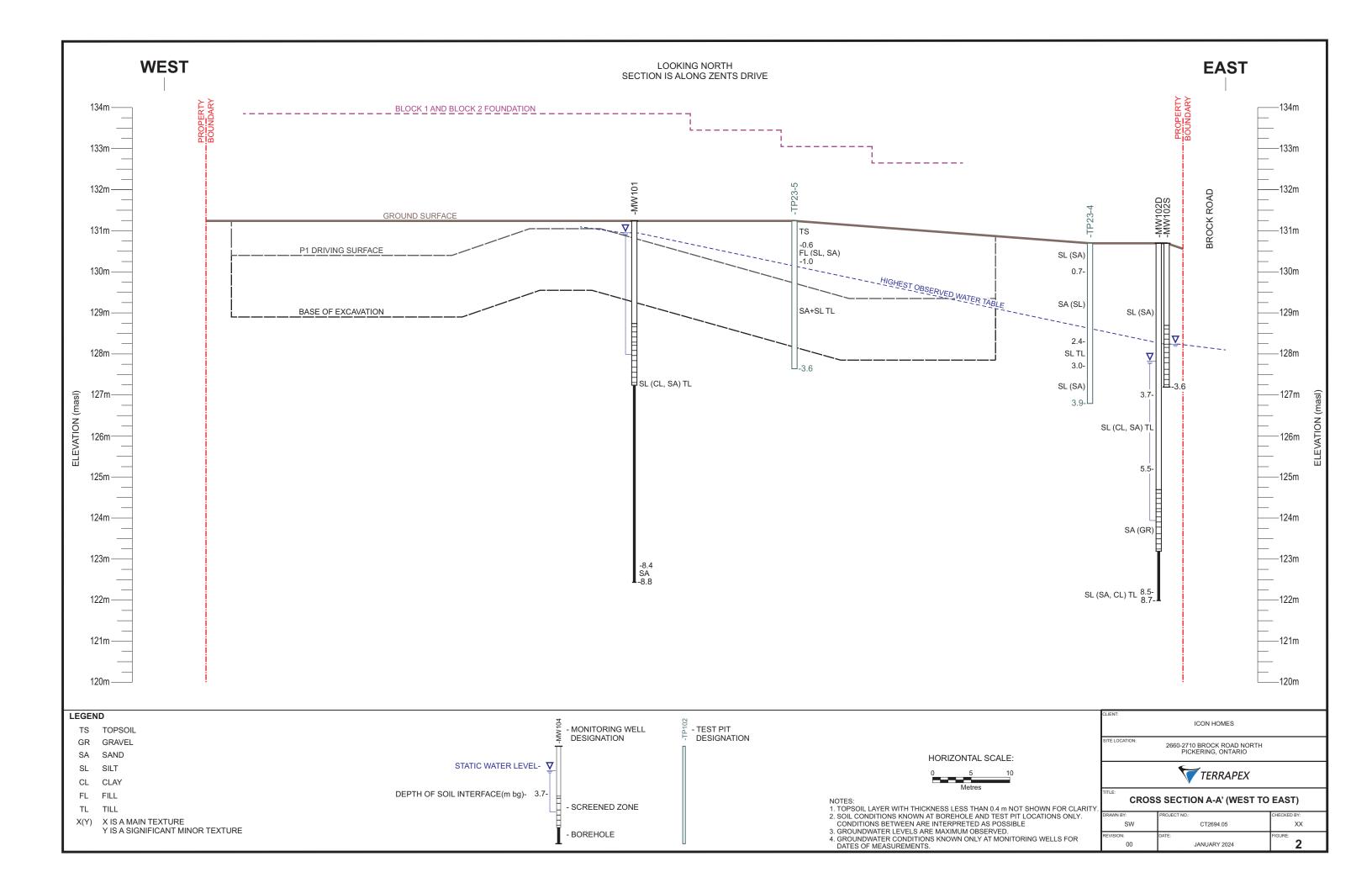
Image: constraint of the second sec	CLIENT: Patheon Developers(Ontario) Inc.				PRO	DJECT I	NO.: C	T269	4.03			R		RD OF:
DONTRACTOR: PONIL METHOD: Solid Service														
BOREHOLE DUMETER (cm) SUPLET SUP # SAULT TYPE BAUGER DRVEN CONTINUE TYPE SUPLET SUP #		ring ON	N NC							-		6.49	ELEV	(m) 131.08
SAMPLE TYPE AUGER DRIVEN CONING OPAMARIZ CONC SHELLEY PUT SPOON SOL E								-		bite S	Spoon			
SOIL DESCRIPTION SOIL (1) (2) (3) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4			t (cm):							П			-	
SOIL DESCRIPTION E E E Offee E OPFEE PUNCL PUN	SAMPLE TYPEAUGERDRIV	en I				V	VATER							I SPOON
TOPSOL 100mm 0 131 1 3 1 3 2 Mat. PHC. PHC. PHC. PHC. PHC. PHC. Barehole dry at competion very sett for hard most. brown -0.5 30.5 - <td< td=""><td></td><td></td><td>ELEVATION (m)</td><td>40 80 12 + N-VAL0 (Blows/300</td><td>20 160 DE Omm)</td><td>PL</td><td>(%) W.C. Ll</td><td>L 80</td><td>SAMPLE NO. SAMPLE TYPE</td><td>RECOVERY (%)</td><td>SV/TOV (ppm or %LEL)</td><td>LABORATORY TESTING</td><td>WELL INSTALLATION</td><td>REMARKS</td></td<>			ELEVATION (m)	40 80 12 + N-VAL0 (Blows/300	20 160 DE Omm)	PL	(%) W.C. Ll	L 80	SAMPLE NO. SAMPLE TYPE	RECOVERY (%)	SV/TOV (ppm or %LEL)	LABORATORY TESTING	WELL INSTALLATION	REMARKS
Ioose, moist, brown molt, brown CLAYEF SANDY SILT trace gravel (TILL) 1 30.5 30.		-0	131 -							-		PAHs,		
TERRAPEX INPUT BY: MW MONITORING DATE:	FILL loose, moist, brown sandy silt very stiff to hard moist, brown CLAYEY SANDY SILT trace gravel (TILL) very dense, moist, grey SANDY SILT trace clay, trace gravel (TILL)	- 1 - 1.5 - 2.5 - 3.5 - 3.5 - 4 - 4.5 - 5.5 - 5.5	130 - 129.5 - 129 - 128.5 - 128 - 127 - 126 - 126 - 125.5 - 125 -		5/6"				2 3 4 6 7 8	98 98 98 98 98 98 98 98 100 100	<5p/1p <5p/1p <5p/1p <5p/1p <5p/1p	M&I, PHCs,		
TERRAPEX INPUT BY: MW MONITORING DATE:														
					LOG	GED BY	: SJ			DRI	LLING [DATE: 0	5-OCt	-2021
	TERRAPEX				INPU	т вү:	MW			мог	NITORI		:	
REVIEWED BY: VN PAGE 1 OF 1	V				REV	EWED	BY: VN	1		PAG	GE 1 OF	1		

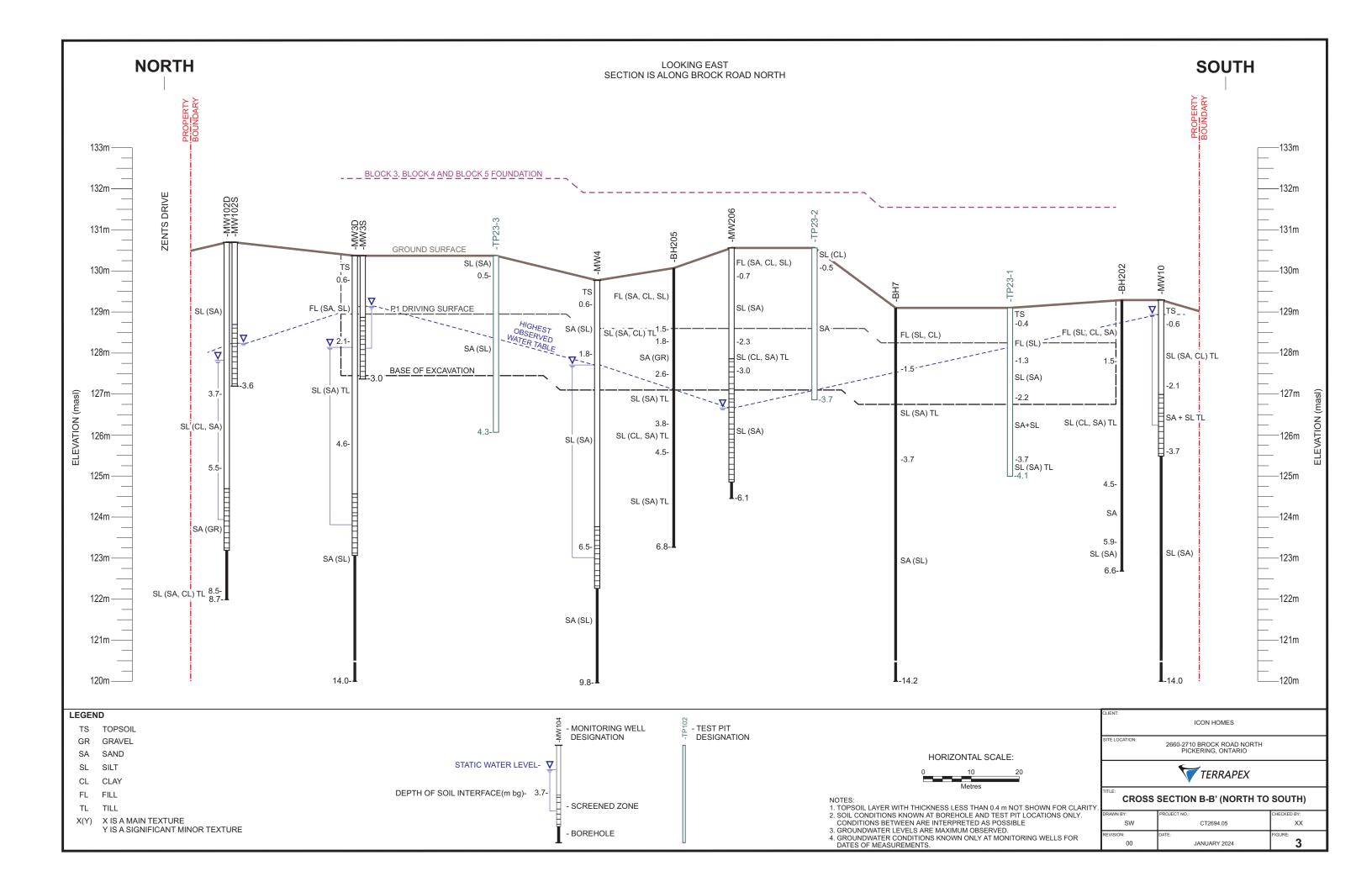
CLIENT: Patheon Developers(Ontario) Inc.				PRC	JECT N	NO.: CT2	2694	.03			R		RD OF:
ADDRESS: 2660-2680 Brock Rd, Pickering C													205
CITY/PROVINCE: 2660-2680 Brock Rd, Picke	ring Ol	N NO	RTHING (m)							65359	2.76	ELEV	. (m) 130.07
CONTRACTOR: Pontil			METH	IOD: S	olid Ste	em Auge	r and	d Sp	ilt S	poon			
BOREHOLE DIAMETER (cm): 16.51 WELL DIA			7	EEN SLO		SAND T			-		_		
	'EN	D	CORING SHEAR STR) ENGTH I		/NAMIC (ATER				SHELB` (new title		SPLI	T SPOON
	DEPTH (m)	ELEVATION (m)	(kPa) 40 80 12 	● 20_160 JE • Dmm)	CC PL	W.C. LL	SAMPLE NO	SAMPLE TYPE		SV/TOV (ppm or %LEL)	ULABORATORY TESTING	WELL INSTALLATION	REMARKS
TOPSOIL 100mm	F 0	130 -	_										
FILL firm, moist, dark brown sandy clayey silt FILL compact, moist, brown sand, trace gravel	- - - - - - - - - - - - - - - - - - -	129.5 - - - - - - - - - - - - - - - - - - -	16				1			5p/0p <5p/0p	PHCs, VOCs PAHs, M&I		Borehole dry at completion
hard, moist, brown CLAYEY SANDY SILT, tr. gravel (TILL dense to very dense moist, brown	- 1.5 	128.5 - 	35 🔺				3/ 3E	\vdash		<5p/1p <5p/1p			
GRAVELLY SAND	127.5		76 •			4		92	5p/1p				
(TILL)	- 3 - - - 3.5 -	127 - - - 126.5 -	52				5		50	5p/1p			
hard, moist, grey CLAYEY SANDY SILT trace gravel (TILL) dense to very dense	- 4 	126 - - - 125.5 -	84	1/6"			6		100	<5p/1p			
wet, grey SANDY SILT trace clay, trace gravel (TILL)	- - - - - 5 - -	125.5		92/6"			7		100	<5p/0p			
	- 5.5 - - - - - -	124.5 - 	43				8		100	<5p/0p			
	- 6.5	123.5 -	7	1			9		100	5p/1p			
END OF BOREHOLE													
TERRAPEX	TERRAPEX										RILLING DATE: 04/5-Oct-2021 DNITORING DATE: GE 1 OF 1		

CLIEN	IT: Patheon Developers(Ontario) Inc.				PRO)JECT I	NO.:	CT26	94.0)3		R		RD OF:
	ESS: 2660-2680 Brock Rd, Pickering Ol													206
	PROVINCE: 2660-2680 Brock Rd, Picker	ing ON	N NC	RTHING (m						-	i): 65363	31.28	ELEV.	(m) 130.56
	RACTOR: Pontil							-			Spoon			
				5.08 SCR						Silica			-	YPE: Bentonite
SAMP	LE TYPE AUGER DRIV	EN I		CORING SHEAR STR			YNAN VATEF		NE I		SHELB (new tit			T SPOON
SOIL SYMBOL	SOIL DESCRIPTION	DEPTH (m)	ELEVATION (m)	(kPa) 40 80 12 	2 <u>0</u> 160		ONTEN (%) W.C.		SAMPLE NO.	SAMPLE TYPE	KECUVEKY (%) SV/TOV (ppm or %LEL)	LABORATORY TESTING	WELL INSTALLATION	REMARKS
SC			ᆸ 130.5 -	20 40 6	0 80	20 4	<u>40 60</u>	80	SA	SA	집양엽		ΝŽ	
	TOPSOIL 70mm FILL, moist, brown, sand and gravel FILL, moist, brown, clayey silty sand trace rootlets	- 0.5	130.5	12					1A 1B		98 <5p/1p <5p/1p	PHCs, VOCs M&I, PAHs		
	compact, moist, brown SANDY SILT	- - - - - -	129.5 -	21					2	1	00<5p/1p			
		- 1.5 - - - 2	129 - 128 5 -	28					3	1	00<5p/1p			
	very stiff, moist, brown CLAYEY SANDY SILT trace gravel	- 2.5	128 -	21					4		6 <5p/1p			
265	(TILL) very dense, moist, grey SANDY SILT occasional layers of clayey silt	- 3 	127.5 -	70/6	•				5		33 < 5p/1p			
		- 	126.5 -	7	2 🔺				6	1	00<5p/1p			
		- 4.5 - - - 5	126 - 125 5 -	7	2				7	1	00<5p/1p			
		- 	125 -	58					8	1	00<5p/1p			
		-6	124.5											
	END OF BOREHOLE													
					LOG	GED BY	/: SJ				RILLING	DATE: 0	5-Oct-2	2021
	TERRAPEX					T BY:				-	ONITORI			
						EWED		/N		-	AGE 1 OF		•	
							J. \			<u>г</u> .				



Attachment C - Hydrostratigraphic Cross Sections (Terrapex, 2024)







Attachment D - Groundwater Level Data (Terrapex, 2022)

TABLE 2Observed Groundwater Levels2660 to 2680 Brock Road and Part of Lot 19, Concession 3;Part 3 and Part 4 on Plan 40R-27228, Pickering, Ontario

Well	Date	Ground	Top Pipe	Well	Ground		Groundwater
Desig.		Elev.	Elev.	Depth	Dep		Elev.
		(m asl)	(m asl)	(m bg)	(m bmp)	(m bg)	(m asl)
MW1(S) Shallow	17-May-18	132.03	133.12	4.70	1.58	0.49	131.54
Snallow	23-May-18 29-May-18				1.76 2.06	0.67 0.96	131.36 131.06
	23-Apr-19				1.29	0.98	131.00
	16-May-19				1.34	0.19	131.78
	19-Jun-19				1.83	0.23	131.30
	26-Jun-19				1.98	0.89	131.14
	02-Jul-19				2.19	1.10	130.93
	26-Apr-21				1.74	0.65	131.38
	13-Oct-21				1.94	0.84	131.18
	18-Oct-21				2.15	1.06	130.97
	27-Oct-21				1.80	0.70	131.33
MW1(D)	17-May-18	132.03	132.88	9.10	5.35	4.50	127.53
Deep	23-May-18				5.19	4.34	127.69
	29-May-18				5.36	4.51	127.53
	23-Apr-19				4.95	4.10	127.94
	16-May-19				4.81	3.96	128.07
	19-Jun-19 26-Jun-19				5.07 5.16	4.22 4.31	127.82 127.73
	02-Jul-19				5.23	4.31	127.65
	26-Apr-21				6.11	4.38 5.26	126.77
	13-Oct-21				6.13	5.28	126.75
	18-Oct-21				6.11	5.26	126.78
	27-Oct-21				6.13	5.28	126.75
MW3(S)	17-May-18	130.34	131.29	3.10	2.15	1.20	129.15
Shallow	23-May-18				2.47	1.52	128.82
	29-May-18				2.72	1.77	128.57
	23-Apr-19				0.07	not moni	
	16-May-19 19-Jun-19				2.87	1.92	128.42
	26-Jun-19				2.73 2.91	1.78 1.96	128.56 128.39
	02-Jul-19				3.02	2.07	128.39
	26-Apr-21				Dry	>3.10	<127.24
	13-Oct-21				Dry	>3.10	<127.24 <127.24
	18-Oct-21				Dry	>3.10	<127.24
	27-Oct-21				Dry	>3.10	<127.24
					,		
MW3(D)	17-May-18	130.37	131.38	7.40	3.53	2.52	127.85
Deep	23-May-18				3.64	2.63	127.74
	29-May-18				3.78	2.77	127.60
	23-Apr-19				2.04	not moni	
	16-May-19				3.24	2.23	128.14
	19-Jun-19				3.61	2.60	127.78
	26-Jun-19				3.69	2.68	127.70
	02-Jul-19				3.75	2.74	127.64
	26-Apr-21 13-Oct-21				4.02	3.01	127.36
	13-Oct-21 18-Oct-21				5.04 6.35	4.03 5.34	126.34 125.04
	27-Oct-21				5.02	5.34 4.01	125.04
					0.02	1.01	120.00

TABLE 2Observed Groundwater Levels2660 to 2680 Brock Road and Part of Lot 19, Concession 3;Part 3 and Part 4 on Plan 40R-27228, Pickering, Ontario

23 29 20 10	Date 7-May-18 3-May-18 9-May-18 3-Apr-19 6-May-19 9-Jun-19	Ground Elev. (m asl) 129.77	Top Pipe Elev. (m asl) 130.74	Well Depth (m bg) 7.60	Ground Dep (m bmp) 3.29		Groundwater Elev. (m asl) 127.45
MW4 17 23 29 2 16	3-May-18 9-May-18 3-Apr-19 6-May-19	(m asl)	(m asl)	(m bg)	(m bmp) 3.29	(m bg)	(m asl)
23 29 20 10	3-May-18 9-May-18 3-Apr-19 6-May-19				3.29		
23 29 20 10	3-May-18 9-May-18 3-Apr-19 6-May-19						1 127.40
29 2 16	9-May-18 3-Apr-19 6-May-19				3.38	2.41	127.37
2 16	3-Apr-19 6-May-19				3.51	2.54	127.23
16	6-May-19				3.11	2.14	127.64
	-				3.04	2.07	127.71
1					3.32	2.35	127.42
2	6-Jun-19				3.41	2.44	127.34
0	2-Jul-19				3.48	2.51	127.27
2	6-Apr-21				4.34	3.37	126.40
1	3-Oct-21				4.52	3.55	126.22
1	8-Oct-21				5.85	4.88	124.90
2	7-Oct-21				4.51	3.54	126.23
MW5 17	7-May-18	131.59	132.52	9.00	6.31	5.37	126.22
23	3-May-18				6.45	5.52	126.07
29	9-May-18				6.69	5.76	125.83
2	3-Apr-19				6.05	5.11	126.48
16	6-May-19				6.20	5.26	126.33
1	9-Jun-19				6.43	5.50	126.09
2	6-Jun-19				6.59	5.65	125.94
0	2-Jul-19				6.70	5.76	125.83
2	6-Apr-21				7.52	6.59	125.00
1	3-Oct-21				7.42	6.48	125.11
1	8-Oct-21				8.90	7.97	123.62
2	7-Oct-21				7.50	6.57	125.02
	7-May-18	131.64	132.57	11.00	5.80	4.87	126.77
	3-May-18				5.94	5.01	126.63
	9-May-18				6.18	5.25	126.39
	3-Apr-19				5.58	4.64	127.00
	6-May-19				5.70	4.76	126.88
	9-Jun-19				5.93	5.00	126.64
	6-Jun-19				6.09	5.15	126.49
	2-Jul-19				6.20	5.26	126.38
	6-Apr-21				6.99	6.06	125.58
	3-Oct-21				6.93	5.99	125.65
	8-Oct-21				8.31 6.94	7.37 6.01	124.27
	7-Oct-21			-			125.63
	9-Jun-19	131.03	132.06	3.96	4.39	3.36	127.67
	6-Jun-19				3.74	2.72	128.32
	2-Jul-19				3.41	2.39	128.65
	6-Apr-21				Dry	>3.96	<127.07
2	7-Oct-21				2.51	1.49	129.55

TABLE 2Observed Groundwater Levels2660 to 2680 Brock Road and Part of Lot 19, Concession 3;Part 3 and Part 4 on Plan 40R-27228, Pickering, Ontario

Well Desig.	Date		Top Pipe	Well	Ground	iwater	Groundwater
		Ground Elev.	Elev.	Depth	Dep		Elev.
		(m asl)	(m asl)	(m bg)	(m bmp)	(m bg)	(m asl)
MW10	17-May-18	129.29	130.28	3.90	2.06	1.08	128.22
	23-May-18	120120	100120	0100	2.25	1.26	128.03
	29-May-18				2.50	1.51	127.79
	23-Apr-19				1.40	0.41	128.89
	16-May-19				1.34	0.35	128.94
	19-Jun-19				1.84	0.85	128.44
	26-Jun-19				1.89	0.90	128.39
	02-Jul-19				2.00	1.01	128,28
	26-Apr-21				1.86	0.87	128.42
	13-Oct-21					inacces	
	18-Oct-21				Dry	>3.90	<125.49
	27-Oct-21				3.37	2.38	126.91
MW101	19-Jun-19	131.24	132.23	4.11	1.35	0.36	130.88
	26-Jun-19				1.50	0.50	130.73
	02-Jul-19				1.73	0.74	130.50
	26-Apr-21				1.28	0.29	130.95
	13-Oct-21				1.93	0.94	130.30
	18-Oct-21				3.55	2.56	128.68
	27-Oct-21				1.82	0.83	130.41
MW102(D)	19-Jun-19	130.70	131.65	7.62	3.84	2.88	127.82
Deep	26-Jun-19				3.90	2.94	127.75
	02-Jul-19				3.97	3.01	127.69
	26-Apr-21				5.21	4.25	126.45
	13-Oct-21				5.26	4.30	126.39
	18-Oct-21				6.62	5.66	125.04
	27-Oct-21				5.24	4.28	126.41
MW102(S)	19-Jun-19	130.68	131.63	3.66	3.39	2.44	128.24
Shallow	26-Jun-19				3.48	2.53	128.15
	02-Jul-19				3.55	2.60	128.08
	26-Apr-21				Dry	>3.66	<127.02
	13-Oct-21				Dry	>3.66	<127.02
	18-Oct-21				Dry	>3.66	<127.02
	27-Oct-21				Dry	>3.66	<127.02
MW203	13-Oct-21	130.65	131.61	6.10	5.96	5.01	125.65
	18-Oct-21				Dry	>6.10	<125.51
	27-Oct-21				5.98	5.03	125.63
MW206	13-Oct-21	129.69	130.56	5.85	4.62	3.75	125.94
	18-Oct-21				4.52	3.65	126.04
	27-Oct-21				4.59	3.72	125.97

Notes

1. Ground elevation interpolated between points on earlier site survey

2. Tops of pipe elevation based on stick up elevation in relation to ground elevation

3. m asl = metres above sea level

4. m bmp = metres below measurement point (Top of pipe)

5. m bg = metres below ground

6. >, < values are based on screen bottom depth and elevation



Attachment E – Single Well Response Tests (Palmer, 2024)

