



HYDROGEOLOGICAL REVIEW REPORT

PREPARED FOR:

UPRC
49 Bogurt Avenue
North York, ON, M2N 1K6

ATTENTION:

Ross Edwards and Edwin Cheng

**1066 Dunbarton Road,
Pickering, Ontario**

Grounded Engineering Inc.

File No. 22-088

Issued August 29, 2022



Executive Summary

Grounded Engineering Inc. (Grounded) was retained by UPRC to conduct a Hydrogeological Review for the proposed redevelopment of 1066 Dunbarton Road in Pickering, Ontario (site). The conclusions of the investigation are summarized as follows:

Site Information

Existing Development					
Site	Above Grade Levels	Below Grade Levels			
		Level #	Lowest Finished Floor		Approximate Base of Foundations (masl)
			Depth (m)	Elevation (masl)	
1066 Dunbarton Rd./ Preliminary	1	1	n/a	n/a	n/a

Proposed Development					
Site	Above Grade Levels	Below Grade Levels			
		Level #	Lowest Finished Floor		Approximate Base of Foundations (masl)
			Depth (m)	Elevation (masl)	
1066 Dunbarton Rd. / Preliminary					
Townhouse 1	3	1	2.8	97.05	96.55
Townhouse 2	3	1	2.8	97.05	96.55
Townhouse 3	3	1	2.8	96.1	95.6
Townhouse 4	3	1	2.8	95.05	94.55
Townhouse 5	3	1	2.8	93.42	92.92

Site Conditions

Site Stratigraphy					
Stratum/Formation	Aquifer or Aquitard	Depth Range (mbgs)	Elevation Range (masl)	Hydraulic Conductivity (m/s)	Method
Fill	Aquifer	0.2 - 0.8	100.0 – 97.9	1.0×10^{-5}	Literature
Silt and Sand	Aquifer	0.8 – 4.6	99.5 – 94.1	5.0×10^{-7}	Literature
Clayey Silt	Aquifer	4.6 – 8.2	95.7 – 90.5	7.21×10^{-9}	Slug Test

Groundwater Elevation	
Design Groundwater Elevation (masl) (see Figure 4)	98.7, 94.0, 95.8



Groundwater Quality

Sample ID	Sample Date	Sample Expiry Date	Region of Durham Storm Sewer Limits	Region of Durham Sanitary Sewer Limits
BH3	May 12, 2022	n/a	Exceeds	Meets

Groundwater Control

Stored Groundwater (pre-excavation/dewatering)

Townhouse ID	Volume of Excavation (m ³)	Volume of Excavation Below Water Table (m ³)	Estimated Volume of Stored Groundwater		Estimated Volume of Available Groundwater	
			m ³	L	m ³	L
1	317	206	200	200,000	100	100,000
2	317	206	200	200,000	100	100,000
3	317	-	-	-	-	-
4	317	-	-	-	-	-
5	317	276	200	200,000	100	100,000

Short Term (Construction) Steady State Groundwater Quantity – Safety Factor of 2.0 Used

Townhouse ID	Estimated Groundwater Seepage		Design Rainfall Event (25mm)		Estimated Total Daily Water Takings	
	L/day	L/min	L/day	L/min	L/day	L/min
1	30,000	20.8	2,880	2.0	32,880	22.8
2	30,000	20.8	2,880	2.0	32,880	22.8
3	-	-	3,000	2.1	3,000	2.1
4	-	-	3,000	2.1	3,000	2.1
5	4,000	2.8	2,880	2.0	6,880	4.8

Long Term (Permanent) Steady State Groundwater Quantity – Safety Factor of 2.0 Used

Townhouse ID	Estimated Groundwater Seepage		Estimated Infiltrated Stormwater – Design Rainfall Event (25mm)		Estimated Total Daily Water Takings	
	L/day	L/min	L/day	L/min	L/day	L/min
1	30,000	20.8	3,000	2.1	33,000	22.9
2	30,000	20.8	3,000	2.1	33,000	22.9
3	-	-	3,000	2.1	3,000	2.1
4	-	-	3,000	2.1	3,000	2.1
5	4,000	2.8	3,000	2.1	7,000	4.9

Land Stability

	Short Term (Construction)	Long Term (Permanent)
Maximum Zone of Influence (m)	29	29
Maximum Potential Settlement (mm)	4	4



Regulatory Requirements	
Environmental Activity and Sector Registry (EASR) Posting	Not Required
Short Term Permit to Take Water (PTTW)	Not Required
Long Term Permit to Take Water (PTTW)	Not Required
Short Term Discharge Agreement City of Pickering / Durham Region	Required
Long Term Discharge Agreement City of Pickering / Durham Region	Required



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FIGURES

- Figure 1 – Study Area Map
- Figure 2 – Borehole Location Plan – Existing
- Figure 3 – Borehole Location Plan – Proposed
- Figure 4 – Subsurface Cross-Section

APPENDICES

- Appendix A – Preliminary Site Grading Plan
- Appendix B – Borehole Logs
- Appendix C – Aquifer Response Tests
- Appendix D – Grain Size Analysis
- Appendix E – HydrogeoSieveXL Data
- Appendix F – Laboratory Certificate of Analysis
- Appendix G – Dewatering Calculations



1 Introduction

Drawings showing the proposed building in plan and profile are currently not available for review. Grounded must be retained to review the architectural drawings prior to construction to ensure that borehole coverage is adequate and that the boreholes are sufficient deep for the proposed development.

UPRC has retained Grounded Engineering Inc. (“Grounded”) to provide hydrogeological engineering design advice for their proposed development at 1066 Dunbarton Road, in Pickering, Ontario.

Property Information	
Location of Site	1066 Dunbarton Road, Pickering, Ontario, L1V 1G8
Ownership of Site	UPRC
Site Dimensions (m)	Approximately 95 m (N/S) x 90 m (E/W)
Site Area (m ²)	Approximately 8,550 m ²

Existing Development	
Number of Building Structures	1
Number of Above Grade Levels	1
Number of Underground Levels	1
Sub-Grade Depth of Development (m)	Approximately 4.0
Sub-Grade Area (m ²)	Approximately 600
Land Use Classification	Commercial

Proposed Development	
Number of Building Structures	5 new building structures
Number of Above Grade Levels	3
Number of Underground Levels	1
Sub-Grade Depth of Development (m)	2.8 m (5 x partial 8m x12m basements only, remainder of building areas will be on-grade)
Sub-Grade Area (m ²)	96 m ² per partial basement (x 5)
Land Use Classification	Residential



Qualified Person and Hydrogeological Review Information

Qualified Person	Kyle Byckalo
Consulting Firm	Grounded Engineering Inc.
Date of Hydrogeological Review	August 29, 2022
Scope of Work	<ul style="list-style-type: none"> ▪ Review of MECP Water Well Records for the area ▪ Review of geological information for the area ▪ Review of topographic information for the area ▪ Advancement of 3 boreholes to a maximum depth of 8 m, which were instrumented with 3 monitoring wells. ▪ Completion of slug tests in all available monitoring wells. ▪ Groundwater elevation monitoring for three (3) months on a bi-weekly basis ▪ Groundwater sampling and analysis to the Region of Durham ▪ Assessment of groundwater controls and potential impacts ▪ Report preparation in accordance with Ontario Water Resources Act, Ontario Regulation 387/04

General Hydrogeological Characterization

Site Topography	The site has an approximate ground surface elevation of 99.5 masl.
Local Physiographic Features	The site is composed of silt and sand overlaying clayey silt deposits.
Regional Physiographic Features	The Iroquois plain on the north shore of Lake Ontario from Scarborough to Newcastle. A sand plain comprised of mosaic till plains and areas of silty lacustrine deposits. The two most important soils of the area are Darlington loam and Newcastle loam, the former developed on the upland areas of till and the latter on the lacustrine sediments in the lower lying areas. They are both good, well drained soils but in some areas can be poorly drained.
Watershed	The site is located within the Lake Ontario Waterfront. Watershed. Locally, groundwater is anticipated to flow south towards Lake Ontario.
Surface Drainage	Surface water is expected to flow towards municipal catch basins located on or adjacent to the site, via Dunbarton Road to the South.



2 Study Area Map

A map has been enclosed which shows the following information:

- All monitoring wells identified on site
- All monitoring wells identified off site within the study area
- All boreholes identified on site
- All buildings identified on site and within the study area
- The Site boundaries
- Any watercourses and drainage features within the study area.

3 Geology and Physical Hydrogeology

The site stratigraphy, including soil materials, composition and texture are presented in detail on the borehole logs in Appendix A. A summary of stratigraphic units that were encountered at the site are as follows:

Site Stratigraphy					
Stratum/Formation	Aquifer or Aquitard	Depth Range (mbgs)	Elevation Range (masl)	Hydraulic Conductivity (m/s)	Method of Determination
Fill	Aquifer	0.2 - 0.8	100.0 – 97.9	1.0×10^{-5}	Literature
Silt and Sand	Aquifer	0.8 – 4.6	99.5 – 94.1	5.0×10^{-7}	Grain Size
Clayey Silt	Aquifer	4.6 – 8.2	95.7 – 90.5	7.21×10^{-9}	Slug Test

Surface Water			
Surface Water Body	Distance from site (m)	Direction from site	Hydraulically Connected to Site (yes/no)
Lake Ontario	Approximately 550	South	no

4 Monitoring Well Information

Well ID	Well Diameter (mm)	Ground Surface (masl)	Top of Screen (masl)	Bottom of Screen (masl)	Screened Geological Unit
BH1	50	98.7	94.2	91.1	Clayey Silt
BH2	50	100.3	95.7	92.7	Clayey Silt



Well ID	Well Diameter (mm)	Ground Surface (masl)	Top of Screen (masl)	Bottom of Screen (masl)	Screened Geological Unit
BH3	50	99.2	94.6	91.6	Clayey Silt

5 Groundwater Elevations

Well ID	Groundwater Elevation (masl)			
	May 12 th , 2022	May 26 th , 2022	June 22 nd , 2022	Maximum
BH1	93.1	93.9	94.0	94.0
BH2	98.7	98.7	98.6	98.7
BH3	93.8	95.5	95.8	95.8

Groundwater levels fluctuate with time depending on the amount of precipitation and surface runoff and may be influenced by known or unknown dewatering activities at nearby sites.

For preliminary design purposes, the design groundwater elevation for each of the proposed townhouse structures is provided in the table below.

Building Structure(s)	Design Groundwater Elevation (m)
Townhouses 1 and 2	98.7
Townhouses 3 and 4	94.0
Townhouse 5	95.8

6 Aquifer Testing

6.1 Single Well Response Test (Slug Test)

The hydraulic conductivities from the monitoring wells were determined based on slug tests (single-well response tests). These tests involve rapid removal of water or addition of a “slug” which displaces a known volume of water from a single well, and then monitoring the water level in the well until it recovers. The results of the slug tests were analyzed using the Bouwer and Rice method (1976).

The hydraulic properties of the strata applicable to the site are as follows:

Well ID	Well Screen Elevation (masl)	Screened Geological Unit	Hydraulic Conductivity (m/s)
BH2	95.7 – 92.7	Clayey Silt	7.21×10^{-9}
BH3	94.6 – 91.6	Clayey Silt	5.05×10^{-9}



6.2 Soil Grain Size Distribution

The hydraulic conductivities of various soil types can also be estimated from grain size analyses. An assessment of the grain sizes was conducted using the excel-based tool, HydrogeoSieve XL (*HydrogeoSieve XL ver.2.2, J.F. Devlin, University of Kansas, 2015*). HydrogeoSieve XL compares the results of the grain size analyses against fifteen (15) different analytical methods.

Given our experience in the area as well as published literature, some of the geometric means provided for the soil were biased low by one or more methods. In these instances, the values determined by these methods were excluded from the mean. The table below illustrates the hydraulic conductivity values estimated from the mean of the analytical methods where the soil met the applicable analysis criteria.

Sample ID	Soil Description	Applicable Analysis Methods	Hydraulic Conductivity (m/s)
BH1-SS3	Sandy Silt, clayey	Alyamani and Sen, Barr, Sauerbrei	4.8×10^{-7}
BH2-SS6	Clay with fines	Alyamani and Sen, Barr, Sauerbrei	1.4×10^{-9}
BH3-SS4	Clay with fines	Alyamani and Sen, Barr, Sauerbrei	1.3×10^{-9}
BH3-SS8	Clay with fines	Alyamani and Sen, Barr, Sauerbrei	1.0×10^{-9}

The results of the analyses are presented in Appendix D.

6.3 Literature

According to Freeze and Cherry (1979), the typical hydraulic conductivity of the strata investigated at the site are:

Stratum/Formation	Hydraulic Conductivity (m/s)
Earth Fill	10^{-2} to 10^{-6}
Silt and Sand	10^{-5} to 10^{-9}
Clayey Silt	10^{-7} to 10^{-10}

7 Water Quality

One (1) unfiltered groundwater sample was collected and analyzed by a Canadian laboratory accredited and licensed by Standards Council of Canada and or Canadian Association for Laboratory Accreditation.

The sample was collected directly from monitoring well BH3 on May 12th, 2022. The sample was analyzed for the following parameters:



- Durham Region Sanitary Sewers Discharge Criteria (55-2013)
- Durham Region Storm Sewers Discharge Criteria (55-2013)

The groundwater sample **exceeded** the **Limits for Storm Sewer Discharge** for the following parameters:

- Total Suspended Solids (Limit 15 mg/L, Result 110 mg/L)

The groundwater sample **met** the **Limits for Sanitary Sewer Discharge** for all parameters analyzed.

A true copy of the analysis report, Certificate of Analysis and a chain of custody record for the sample are enclosed.

8 Proposed Construction Method

The proposed methodology to support open excavations at the site is currently undetermined. For the purposes of this report, equivalent well analyses were conducted employing conventional open cut excavation in order to determine a “worst-case scenario” with respect to dewatering volumes and groundwater seepage at the site.

For design purposes, the groundwater table is at about Elev. 98.7± masl for Townhouses 1 and 2 in the silt and sand deposits, at Elev. 94.0± masl for Townhouses 3 and 4 in the clayey silt deposit, and at Elev. 95.8± masl for Townhouse 5 also in the clayey silt deposit. These deposits have a relatively low permeability and will yield only minor seepage in the long term. The groundwater table is present in all of the native soil units.

The lowest (B1) FFE for all Townhouses 1 and 2 is at Elev. 97.05± m, Townhouse 3 is at Elev. 96.1± m, Townhouse 4 is at Elev. 95.05± m and Townhouse 5 is at Elev. 93.42± m. Therefore,

- Bulk excavations for basements will extend below the elevation of the design groundwater table(s) for Townhouses 1, 2, and 5.
- Foundation excavations will be made in relatively low-permeability soils below the design groundwater table.
- Bulk excavations for basements will remain above the design groundwater table(s) for Townhouses 3 and 4.
- Foundation excavations may extend below the design groundwater table, however they will be made in relatively low-permeability soils.

Dewatering of all basement excavations will be required to facilitate construction as well as to maintain the integrity of the subgrade for foundation and slab-on-grade support. The water level must be kept at least 1.2 m below the lowest excavation elevation during construction. Failure to dewater prior to final excavation may result in unrecoverable disturbance of the subgrade, which will render advice provided for undisturbed subgrade conditions inapplicable.



Stored water within the excavation will need to be considered prior to excavation/dewatering.

If it is proposed to discharge groundwater to the municipal sewers, a professional dewatering contractor must be consulted to review the subsurface conditions and to design a site-specific dewatering system. It is the dewatering contractor's responsibility to assess the factual data and to provide recommendations on dewatering system requirements.

9 Private Water Drainage System (PWDS)

If the proposed development consists of drained foundations, then a private water drainage system will be required for structures with basements constructed below the groundwater table. The total sub floor drain area is approximately 96 m² per basement based on an excavated area of 12 m x 8 m.

If basements are designed with a private water drainage system, the drainage system is a critical structural element since it keeps water pressure from acting on the basement walls and floor slab. As such, the sump that ensures the performance of this system must have a duplexed pump arrangement for 100% pumping redundancy and these pumps must be on emergency power. The size of the sump should be adequate to accommodate the estimated groundwater seepage. It is anticipated that the groundwater seepage can be controlled with typical, widely available, commercial/residential sump pumps.

If the proposed basements are designed as watertight structures, then a private water drainage system will not be required. However, the structure must then be designed to resist hydrostatic pressure and uplift forces.

10 Groundwater Extraction and Discharge

Equivalent well analyses were conducted for both short-term and long-term dewatering scenarios. Equivalent well analyses for groundwater seepage indicates the short term (construction) and long term (permanent) dewatering requirements as provided below. The results are presented in Appendix G.

The groundwater seepage estimates, which have been provided, represent the steady state groundwater seepage. There will be an initial drawdown of the groundwater before a steady state condition is reached. The rate of the initial drawdown, and therefore discharge, is dependent on the dewatering contractor and how the groundwater is being dealt with at the site. An estimated initial volume of stored groundwater which will require removal before steady state is reached has been provided below.

Please note that if excavation is exposed to the elements, stormwater will have to be managed. The short-term control of groundwater should consider stormwater management from rainfall events. A dewatering system should be designed to consider the removal of rainfall from excavation. A design storm of 25 mm has been used in the quantity estimates.



As required by Ontario Regulation 63/16, a plan for discharge must consider the conveyance of stormwater from a 100-year storm. The additional volume that will be generated in the occurrence of a 100-year storm event is approximately 10,000 L.

The following design considerations and values have been incorporated into the numerical modelling / dewatering estimates:

- A Factor of Safety of 2.0 was used for all groundwater seepage volume calculations.
- The design hydraulic conductivities for the site are:

Design Hydraulic Conductivity	
Stratum/Formation	K (m/s)
Earth Fill	1.0×10^{-5}
Silt and Sand	5.0×10^{-7}
Clayey Silt	7.21×10^{-9}

Stored Groundwater (pre-excavation/dewatering)						
Townhouse ID	Volume of Excavation (m ³)	Volume of Excavation Below Water Table (m ³)	Estimated Volume of Stored Groundwater		Estimated Volume of Available Groundwater	
			m ³	L	m ³	L
1	317	206	200	200,000	100	100,000
2	317	206	200	200,000	100	100,000
3	317	-	-	-	-	-
4	317	-	-	-	-	-
5	317	317	276	200	200,000	100

Short Term (Construction) Steady State Groundwater Quantity – Safety Factor of 2.0 Used						
Townhouse ID	Estimated Groundwater Seepage		Design Rainfall Event (25mm)		Estimated Total Daily Water Takings	
	L/day	L/min	L/day	L/min	L/day	L/min
1	30,000	20.8	2,880	2.0	32,880	22.8
2	30,000	20.8	2,880	2.0	32,880	22.8
3	-	-	3,000	2.1	3,000	2.1
4	-	-	3,000	2.1	3,000	2.1
5	4,000	2.8	2,880	2.0	6,880	4.8



Long Term (Permanent) Steady State Groundwater Quantity – Safety Factor of 2.0 Used						
Townhouse ID	Estimated Groundwater Seepage	Estimated Infiltrated Stormwater – Design Rainfall Event (25mm)		Estimated Total Daily Water Takings		
	L/day	L/min	L/day	L/min	L/day	L/min
1	30,000	20.8	3,000	2.1	33,000	22.9
2	30,000	20.8	3,000	2.1	33,000	22.9
3	-	-	3,000	2.1	3,000	2.1
4	-	-	3,000	2.1	3,000	2.1
5	4,000	2.8	3,000	2.1	7,000	4.9

Regulatory Requirements	
Environmental Activity and Sector Registry (EASR) Posting	Not Required
Short Term Permit to Take Water (PTTW)	Not Required
Long Term Permit to Take Water (PTTW)	Not Required
Short Term Discharge Agreement City of Pickering / Durham Region	Required
Long Term Discharge Agreement City of Pickering / Durham Region	Required

Please note:

- The native soils must be dewatered a minimum of 1.2 m below the footing elevation prior to foundation excavation to preserve the in-situ integrity of the native soils during construction dewatering activities.
- The proposed pump schedule for short term construction dewatering has not been completed. As such, the actual peak short term discharge rate is not available at the time of writing this report. The pump schedule must be specified by either the dewatering contractor retained or the mechanical consultant.
- The proposed pump schedule for long term permanent drainage has not been completed. As such the actual peak long term discharge rate is not available at the time writing of this report. The pump schedule must be specified by the mechanical consultant if necessary.
- Watertight basement structures (that do not include a private water drainage system) have not been considered as part of the proposed development at this time.
- On-site containment (infiltration gallery/dry well etc.) has not been considered as part of the proposed development at this time. If this option is considered, additional work will have to be conducted (i.e. infiltration testing).



11 Evaluation of Impact

11.1 Zone of Influence (ZOI)

The Zone of Influence (ZOI) with respect to groundwater was calculated based on the estimated groundwater taking rate and the hydraulic conductivity of the unit which water will be taken at the Site.

The ZOI was calculated using the Sichardt equation below.

Equation:

$$R_0 = 3000(\Delta H)\sqrt{K}$$

ΔH = dewatering thickness (m)
 K = hydraulic conductivity (m/s)
 R_0 = radius of influence (m)

The ZOI with respect to groundwater seepage at the site is summarized as follows.

Zone of Influence (ZOI)		
	Short Term (Construction)	Long Term (Permanent)
Maximum Zone of Influence (m)	29	29

11.2 Land Stability

The impacts to land stability on adjacent structures due to the proposed short- and long-term dewatering at the site are summarized as follows:

Land Stability		
	Short Term (Construction)	Long Term (Permanent)
Dewatering Thickness (m)	4.6	4.6
Increase in Effective Stress (kPa)	45	45
Maximum Theoretical Settlement due to Dewatering (mm)	4	4
Public Realm Theoretical Settlement due to Dewatering (mm)	4	4

The theoretical maximum induced settlement occurs directly adjacent to the proposed excavation and decreases in a nonlinear fashion with distance away from the excavation.

On this basis, the impact of the proposed dewatering on the existing adjacent structures is considered by Grounded to be within acceptable limits.



11.3 City's Sewage Works

Negative impacts to City's sewage works may occur in terms of the quantity or quality of the groundwater discharged. This report provided the estimated quantity of the water discharge. However, this report does not speak to the sewer capacities. The sewer capacity analysis is provided under a separate cover by the civil consultant.

The quality of the proposed groundwater discharge is provided in Section 7. As noted in that section, the groundwater sample exceeded the Limits for Storm Sewer Discharge and met the Limits for Sanitary Sewer Discharge.

As such, additional treatment will be required before the water can be discharged to the Storm Sewer to avoid impacts to the City's sewage works caused by groundwater quality. Additional treatment will not be required before the water can be discharged to the Sanitary and Combined Sewer.

11.4 Natural Environment

There are no natural waterbodies within the ZOI that will be affected by the proposed construction dewatering or permanent drainage. Any groundwater which will be taken from the site will be discharged (if required) into the City's sewer systems and not into any natural waterbody. As such, there will be no impact to the natural environment caused by the water takings at the site.

11.5 Local Drinking Water Wells

The site is located within the municipal boundaries of the City of Pickering. The site and surrounding area are provided with municipal piped water and sewer supply. There is no use of the groundwater for water supply in this area of Toronto. As such, there will be no impact to drinking water wells.

11.6 Contamination Source

The site and immediately surrounding area currently consist mostly of residential and commercial areas. These land uses are not anticipated to be a source of potential contamination and are not expected to provide an Area of Potential Environmental Concern for the site. As such, the pumping of groundwater at the site is not anticipated to facilitate the movement of potential contaminants onto the site. Evaluation of the environmental condition of the site has been completed under a separate cover.



12 Proposed Mitigation Measures and Monitoring Plan

The extent of the negative impact identified in previous sections will be limited to the ZOI caused by the groundwater taking at the site.

As a result of dewatering and draining the soil, changes in groundwater level have the potential to cause settlement based on the change in the effective stresses within the ZOI.

If adjacent buildings or municipal infrastructure are within the ZOI and will undergo settlement that may be considered unacceptable as identified the Land Stability Section, consideration should be given to implement a monitoring and mitigation program during dewatering activities.

Both the temporary construction dewatering system and the permanent building drainage system must be properly installed and screened to ensure sediments and fines will not be removed, which is typically a primary cause of dewatering related settlement.

13 Limitations

Natural occurrences, the passage of time, local construction, and other human activity all have the potential to directly or indirectly alter the subsurface conditions at or near the project site. Contractual obligations related to groundwater or stormwater control must be considered with attention and care as they relate this potential site alteration.

The hydrogeological engineering advice provided in this report is based on the factual observations made from the site investigations as reported. It is intended for use by the owner and their retained design team. If there are changes to the features of the development or to the scope, the interpreted subsurface information, geotechnical engineering design parameters, advice, and discussion on construction considerations may not be relevant or complete for the project. Grounded should be retained to review the implications of such changes with respect to the contents of this report.

Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. Grounded accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report, including consequential financial effects on transactions or property values, or requirements for follow-up actions and costs.

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Drawings showing the proposed building in plan and profile are currently not available for review. Grounded must be retained to review the architectural drawings prior to construction to ensure



that borehole coverage is adequate and that the boreholes are sufficient deep for the proposed development.

14 Closure

If there are any questions regarding the discussion and advice provided, please do not hesitate to contact our office. We trust that this report meets your requirements at present.

For and on behalf of our team,





S. BASTAN
100532430
2022-08-29
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Sam Bastan, P. Eng.,
Project Engineer



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100199873
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PROVINCE OF ONTARIO

Kyle Byckalo, P. Eng.,
Senior Project Engineer

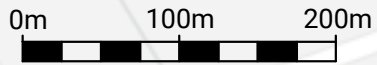
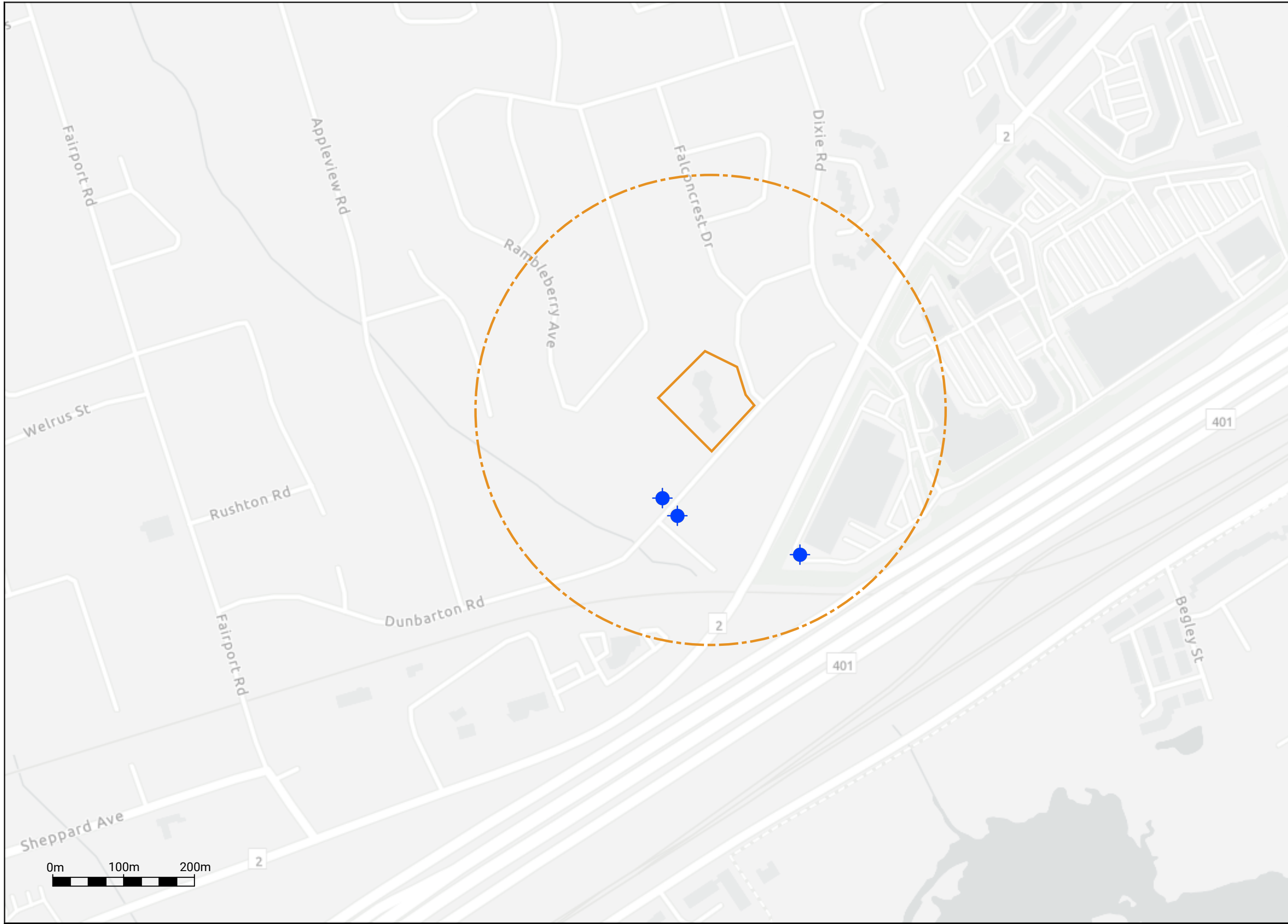


M. J. BIELASKI
100131738
2022.08.30
PROVINCE OF ONTARIO

Matt Bielaski, P. Eng.
Principal

FIGURES





GROUND
ENGINEERING

1 BANIGAN DRIVE, TORONTO, ONT., M4H 1G3
www.groundedeng.ca

LEGEND

- APPROXIMATE PROPERTY BOUNDARY
- STUDY AREA (250 m RADIUS)
- MECP WELL LOCATION

Note

Reference

ArcGIS MyMaps 2022.

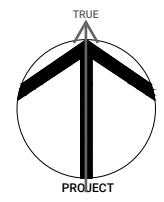
Project

**1066 DUNBARTON ROAD,
PICKERING, ONTARIO**

Figure Title

SITE LOCATION PLAN

North



Date

AUGUST 2022

Scale

AS INDICATED

Job No

22-088

Figure No

FIGURE 1

LEGEND

— APPROXIMATE PROPERTY BOUNDARY

⊕ BOREHOLE WITH MONITORING WELL

▲ SECTION LOCATION

Note

Reference

Survey Drawing Job No.: 220-0021
Dated: Apr. 22, 2022
Prepared by: Speight, Van Nortrand & Gibson Limited.

Project

**1066 DUNBARTON ROAD,
PICKERING, ONTARIO**

Figure Title

**BOREHOLE LOCATION
PLAN - EXISTING**

North



Date

AUGUST 2022

Scale

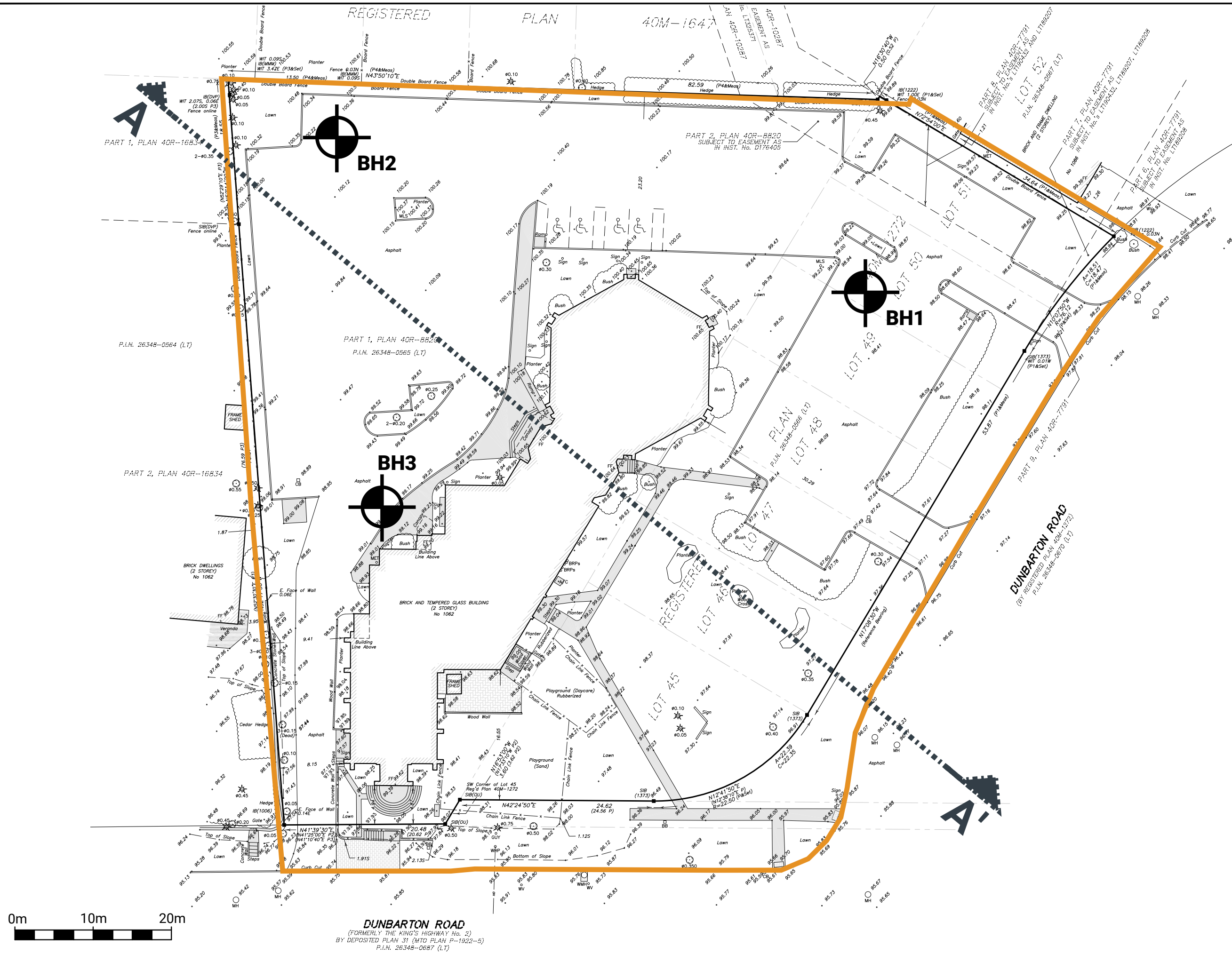
AS INDICATED

Job No

22-088

Figure No

FIGURE 2



DUNBARTON ROAD
(FORMERLY THE KING'S HIGHWAY No. 2)
BY DEPOSITED PLAN 31 (MTO PLAN P-1922-5)
P.I.N. 26348-0687 (LT)



GROUND
ENGINEERING

1 BANIGAN DRIVE, TORONTO, ONT., M4H 1G3
www.groundedeng.ca

LEGEND

- APPROXIMATE PROPERTY BOUNDARY
- ⊕ MONITORING WELL LOCATION

Note

Reference

Conceptual Site Plan A1-03
Prepared by KBMB Architects
Dated 05/04/22
Received 06/30/22

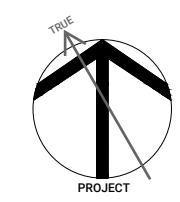
Project

**1066 DUNBARTON ROAD,
PICKERING, ONTARIO**

Figure Title

**BOREHOLE LOCATION
PLAN - PROPOSED**

North



Date

AUGUST 2022

Scale

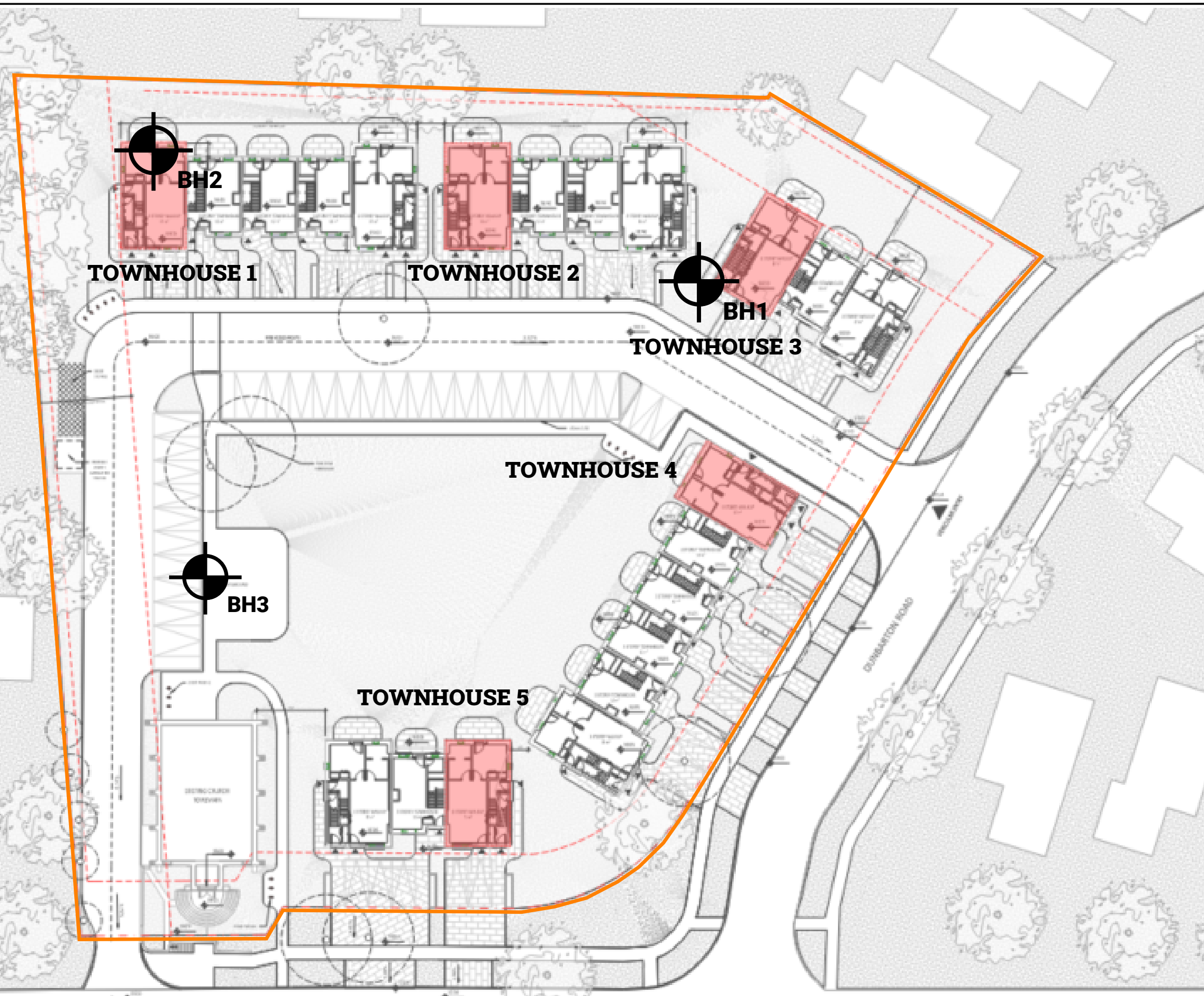
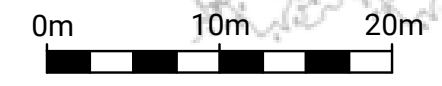
AS INDICATED

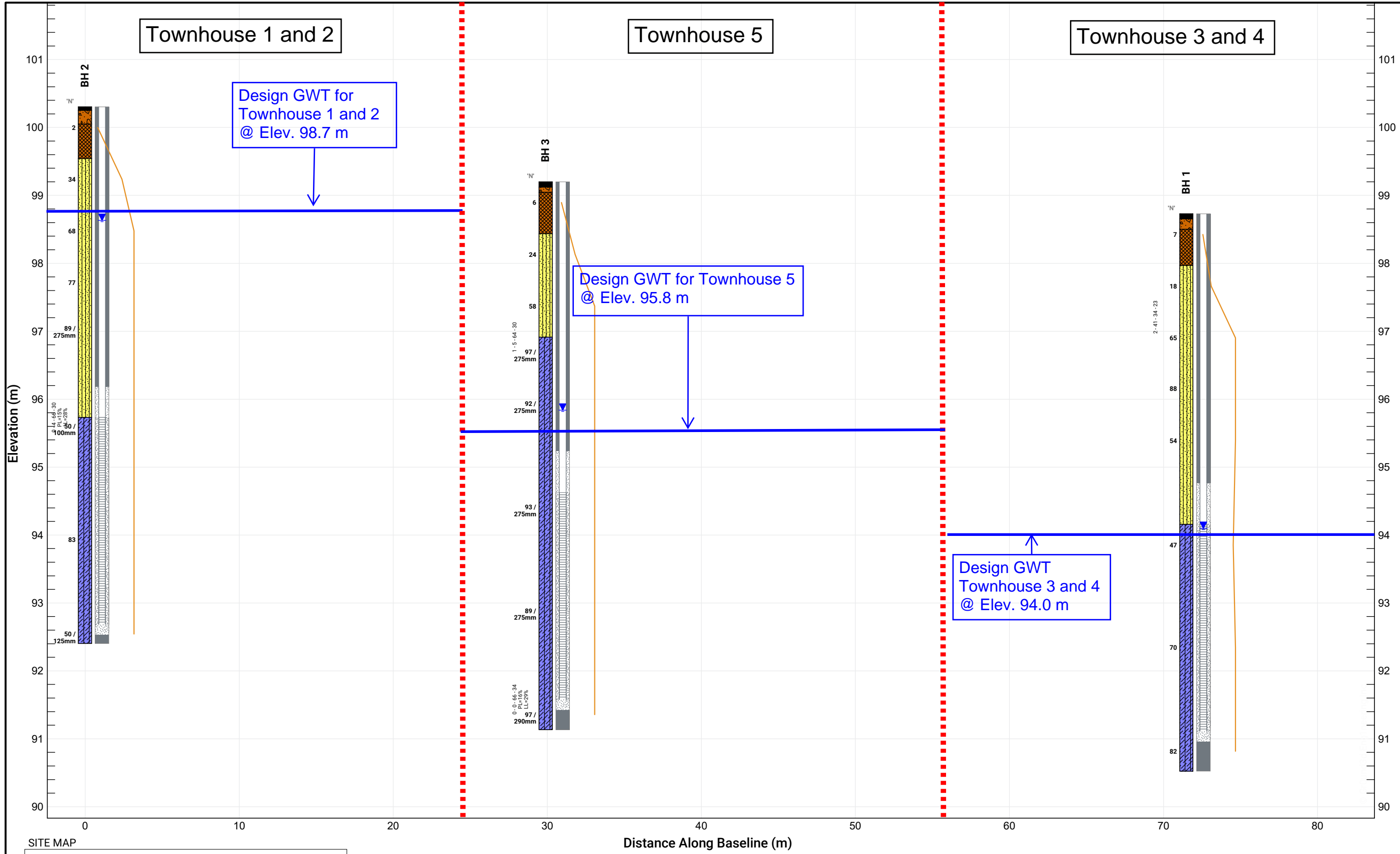
Job No

22-088

Figure No

FIGURE 3





LEGEND

- FILL
- GRAVELS (gravel to gravelly sand)
- SILT TO SAND (not till)
- COHESIONLESS TILLS
- COHESIVE SOILS (clayey silt to clay, incl. tills)
- DISTURBED/REWORKED/ORGANIC

▽ water level, unstabilized
 ▽ water level, stabilized (latest)
 ▽ water level, stabilized (highest)

Project
**1066 DUNBARTON RD.
PICKERING, ON**

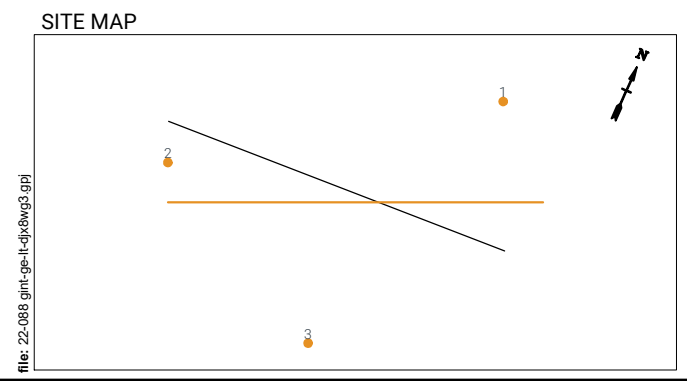
Figure Title
SUBSURFACE PROFILE

Date
AUGUST 2022

Scale
AS INDICATED

Job No
22-088

Figure No
FIGURE 4

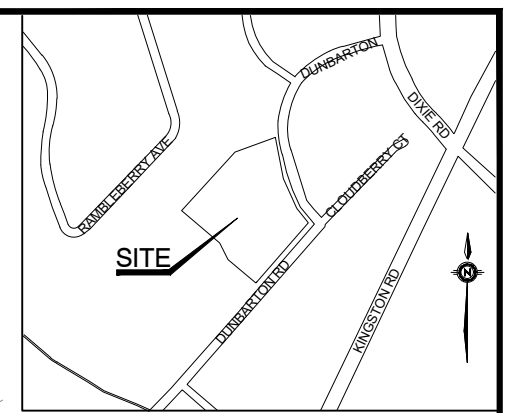


BOREHOLE STRATIGRAPHY LEGEND

- Asphalt
- Aggregate
- Fill
- Silt and Sand
- Clayey Silt

APPENDIX A





KEY PLAN NTS

Preliminary
2022-08-05 2:01:17 PM

- LEGEND
- SITE BOUNDARY
 - SWALE
 - + 170.72 PROPOSED ELEVATION
 - HP HIGH POINT
 - LP LOW POINT
 - +EX.167.16 EXISTING ELEVATION
 - +SW.167.16 SWALE ELEVATION
 - DIRECTION OF OVERLAND FLOW
 - STORMWATER CATCHBASIN

CLIENT
UNITED PROPERTY RESOURCE CORPORATION

TITLE
1066 DUNBARTON ROAD
PRELIMINARY SITE GRADING PLAN

100 Scotia Court
Whitby, ON L1N 8Y6
t. 905.668.3022
f. 905.668.9443
www.wsp.com

Checked	M.I.	Drawn	N.M.M
Date	AUGUST 2022	Proj. No.	221-05497
Scale	1:500	Figure No.	4

Aug 05, 2022 - 2:00pm, CAN070622
 C:\Users\CAN070622\OneDrive\Work\Projects\1066 Dunbarton Road\1066 Dunbarton Road - PREGR.dwg - tab:Fig 4 - PRE GR

APPENDIX B



SAMPLING/TESTING METHODS

SS: split spoon sample
 AS: auger sample
 GS: grab sample
 FV: shear vane
 DP: direct push
 PMT: pressuremeter test
 ST: shelby tube
 CORE: soil coring
 RUN: rock coring

SYMBOLS & ABBREVIATIONS

MC: moisture content
 LL: liquid limit
 PL: plastic limit
 PI: plasticity index
 γ : soil unit weight (bulk)
 G_s : specific gravity
 S_u : undrained shear strength
 unstabalized water level
 1st water level measurement
 2nd water level measurement most recent
 water level measurement

ENVIRONMENTAL SAMPLES

M&I: metals and inorganic parameters
 PAH: polycyclic aromatic hydrocarbon
 PCB: polychlorinated biphenyl
 VOC: volatile organic compound
 PHC: petroleum hydrocarbon
 BTEX: benzene, toluene, ethylbenzene and xylene
 PPM: parts per million

FIELD MOISTURE (based on tactile inspection)

DRY: no observable pore water
MOIST: inferred pore water, not observable (i.e. grey, cool, etc.)
WET: visible pore water

COHESIONLESS

Relative Density	N-Value
Very Loose	<4
Loose	4 - 10
Compact	10 - 30
Dense	30 - 50
Very Dense	>50

COHESIVE

Consistency	N-Value	Su (kPa)
Very Soft	<2	<12
Soft	2 - 4	12 - 25
Firm	4 - 8	25 - 50
Stiff	8 - 15	50 - 100
Very Stiff	15 - 30	100 - 200
Hard	>30	>200

COMPOSITION

Term	% by weight
trace silt	<10
some silt	10 - 20
silty	20 - 35
sand and silt	>35

ASTM STANDARDS

ASTM D1586 Standard Penetration Test (SPT)

Driving a 51 mm O.D. split-barrel sampler ("split spoon") into soil with a 63.5 kg weight free falling 760 mm. The blows required to drive the split spoon 300 mm ("bpf") after an initial penetration of 150 mm is referred to as the N-Value.

ASTM D3441 Cone Penetration Test (CPT)

Pushing an internal still rod with a outer hollow rod ("sleeve") tipped with a cone with an apex angle of 60° and a cross-sectional area of 1000 mm² into soil. The resistance is measured in the sleeve and at the tip to determine the skin friction and the tip resistance.

ASTM D2573 Field Vane Test (FVT)

Pushing a four blade vane into soil and rotating it from the surface to determine the torque required to shear a cylindrical surface with the vane. The torque is converted to the shear strength of the soil using a limit equilibrium analysis.

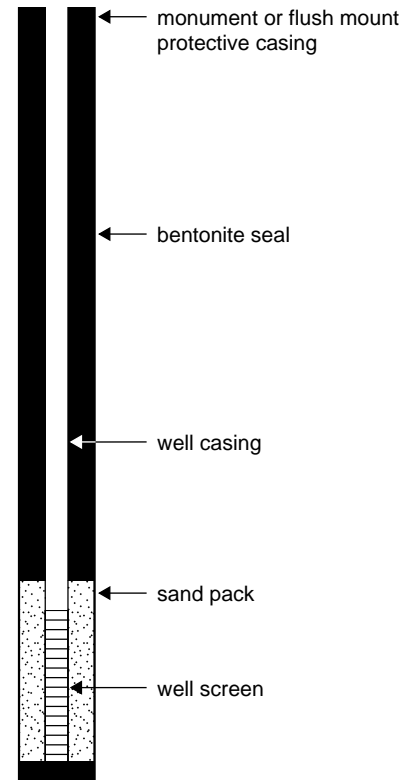
ASTM D1587 Shelby Tubes (ST)

Pushing a thin-walled metal tube into the in-situ soil at the bottom of a borehole, removing the tube and sealing the ends to prevent soil movement or changes in moisture content for the purposes of extracting a relatively undisturbed sample.

ASTM D4719 Pressuremeter Test (PMT)

Place an inflatable cylindrical probe into a pre-drilled hole and expanding it while measuring the change in volume and pressure in the probe. It is inflated under either equal pressure increments or equal volume increments. This provides the stress-strain response of the soil.

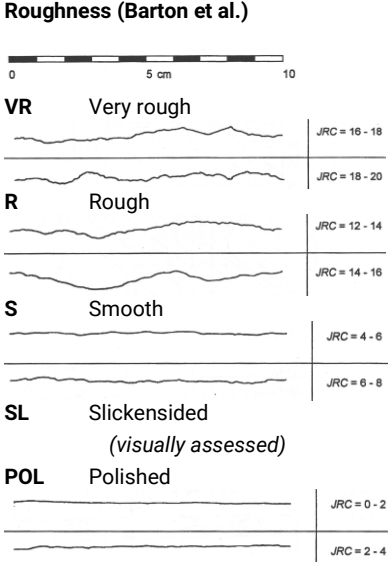
WELL LEGEND



- TCR Total Core Recovery** the total length of recovery (soil or rock) per run, as a percentage of the drilled length
- SCR Solid Core Recovery** the total length of sound full-diameter rock core pieces per run, as a percentage of the drilled length
- RQD Rock Quality Designation** the sum of all pieces of sound rock core in a run which are 10 cm or greater in length, as a percentage of the drilled length

Natural Fracture Frequency (typically per 0.3 m) The number of natural discontinuities (joints, faults, etc.) which are present per 0.3m. Ignores mechanical or drill-induced breaks, and closed discontinuities (e.g. bedding planes).

LOGGING DISCONTINUITIES

<p>Discontinuity Type</p> <p>BP bedding parting CL cleavage CS crushed seam FZ fracture zone MB mechanical break IS infilled seam JT Joint SS shear surface SZ shear zone VN vein VO void</p> <p>Coating</p> <p>CN Clean SN Stained OX Oxidized VN Veneer CT Coating (>1 mm)</p> <p>Dip Inclination</p> <p>H horizontal/flat 0 - 20° D dipping 20 - 50° SV sub-vertical 50 - 90° V vertical 90±°</p>	<p>Roughness (Barton et al.)</p>  <p>VR Very rough JRC = 16 - 18</p> <p>R Rough JRC = 12 - 14</p> <p>S Smooth JRC = 14 - 16</p> <p>SL Slickensided (visually assessed) JRC = 6 - 8</p> <p>POL Polished JRC = 0 - 2</p>	<p>Spacing in Discontinuity Sets (ISRM 1981)</p> <p>VC very close < 60 mm C close 60 - 200 mm M mod. close 0.2 to 0.6 m W wide 0.6 to 2 m VW very wide > 2 m</p> <p>Aperture Size</p> <p>T closed / tight < 0.5 mm GA gapped 0.5 to 10 mm OP open > 10 mm</p> <p>Planarity</p> <p>PR Planar UN Undulating ST Stepped IR Irregular DIS Discontinuous CU Curved</p>
---	--	---

GENERAL

Degree of Weathering (after MTO, RR229 Evaluation of Shales for Construction Projects)

Zone	Degree	Description
Z1	unweathered	shale, regular jointing
Z2	partially weathered	angular blocks of unweathered shale, no matrix, with chemically weathered but intact shale
Z3		soil-like matrix with frequent angular shale fragments < 25mm diameter
Z4a		soil-like matrix with occasional shale fragments < 3mm diameter
Z4b	fully weathered	soil-like matrix only

Strength classification (after Marinis and Hoek, 2001; ISRM 1981b)

Grade	UCS (MPa)	Field Estimate (Description)
R6	extremely strong > 250	can only be chipped by geological hammer
R5	very strong 100 - 250	requires many blows from geological hammer
R4	strong 50 - 100	requires more than one blow from geological hammer
R3	medium strong 25 - 50	can't be scraped, breaks under one blow from geological hammer
R2	weak 5 - 25	can be peeled / scraped with knife with difficulty
R1	very weak 1 - 5	easily scraped / peeled, crumbles under firm blow of geo. hammer
R0	extremely weak < 1	indented by thumbnail

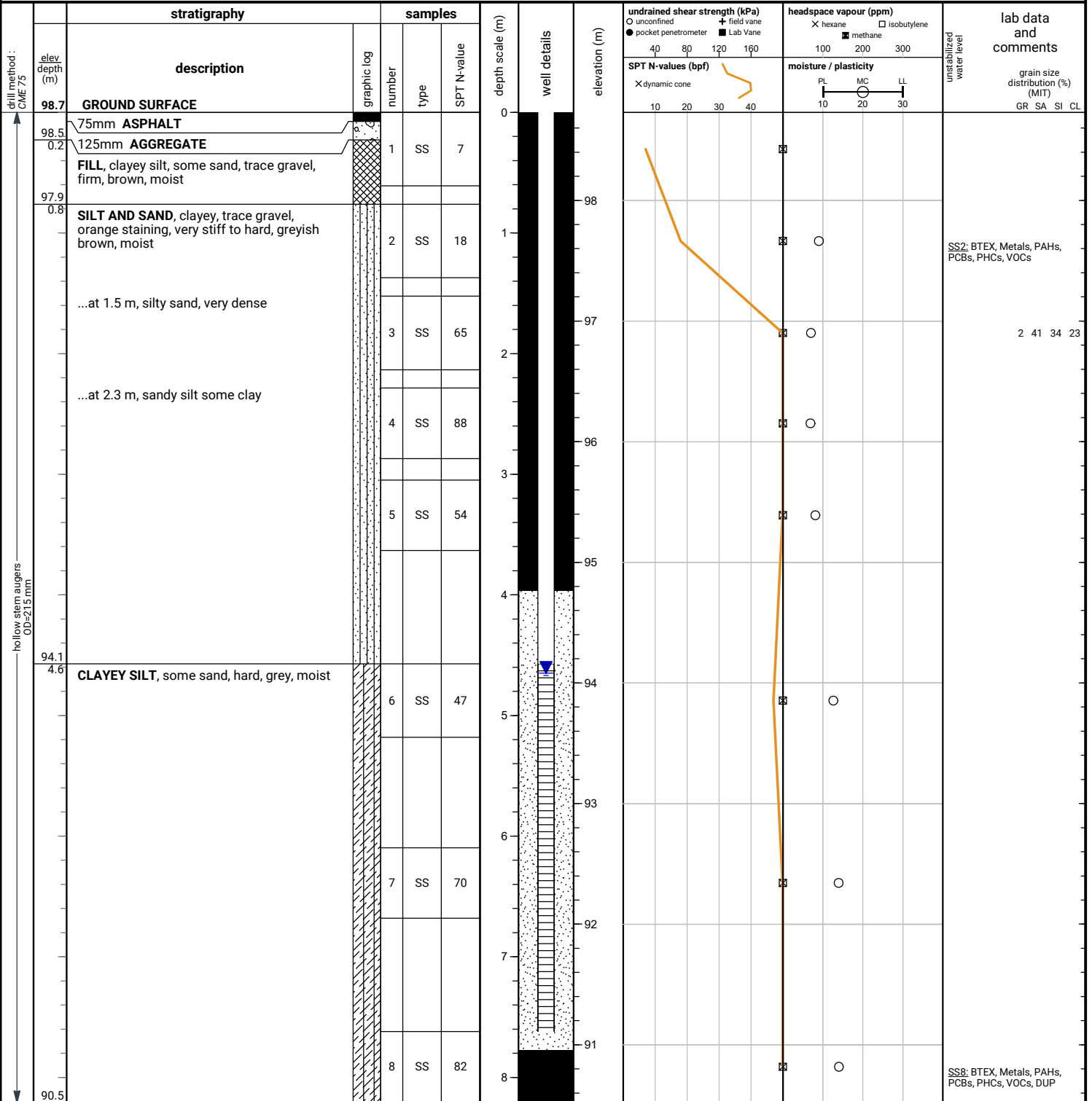
Bedding Thickness (Q. J. Eng. Geology, Vol 3, 1970)

Very thickly bedded	> 2 m
Thickly bedded	0.6 - 2m
Medium bedded	200 - 600mm
Thinly bedded	60 - 200mm
Very thinly bedded	20 - 60mm
Laminated	6 - 20mm
Thinly Laminated	< 6mm

File No. : 22-088

Project : 1066 Dunbarton Rd., Pickering, ON

Client : UPRC c/o Turner & Townsend



END OF BOREHOLE

Dry and open upon completion of drilling.
 50 mm dia. monitoring well installed.
 No. 10 screen

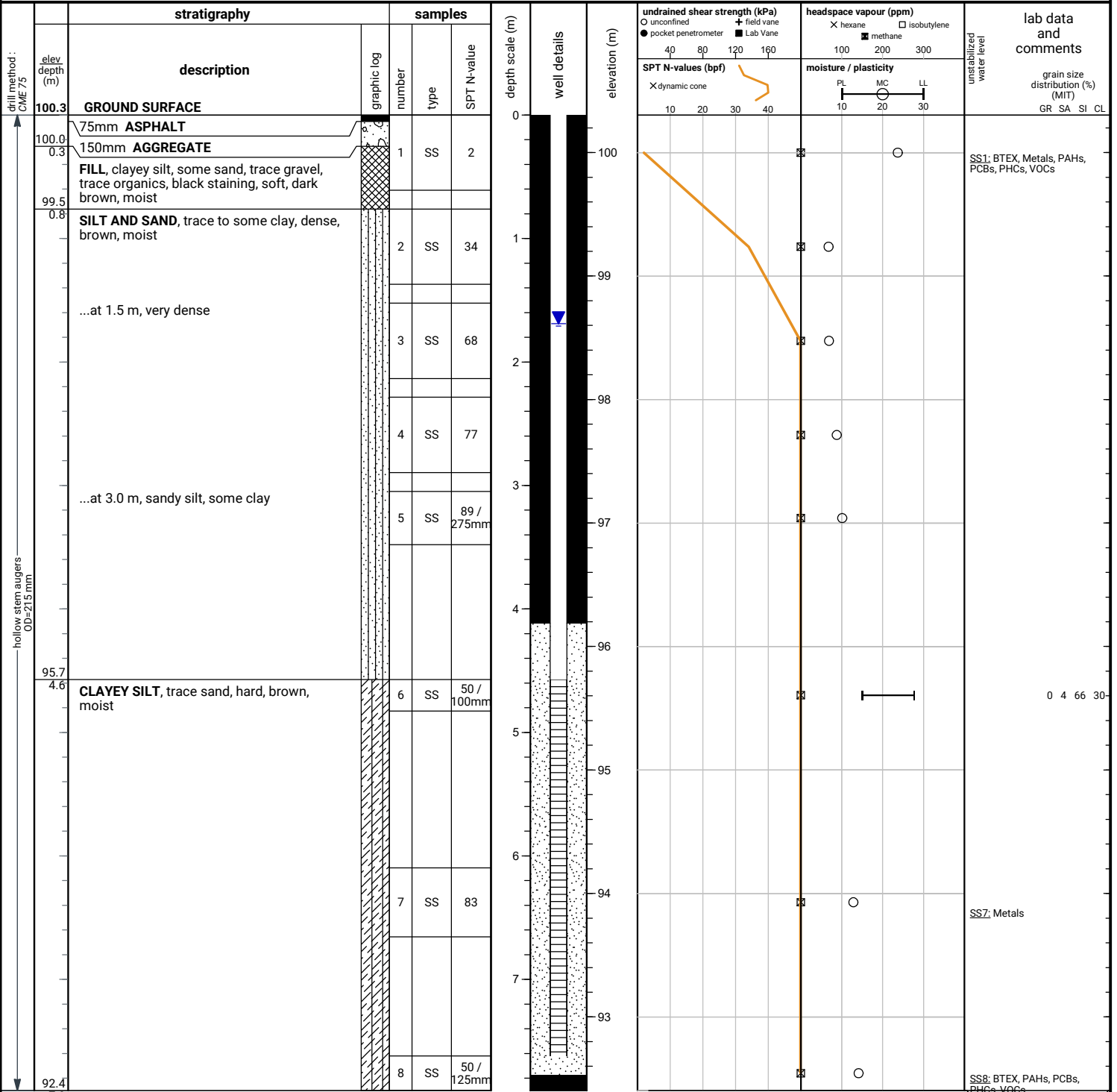
GROUNDWATER LEVELS

date	depth (m)	elevation (m)
May 12, 2022	5.6	93.1
May 26, 2022	4.8	93.9
Jun 22, 2022	4.7	94.0

File No. : 22-088

Project : 1066 Dunbarton Rd., Pickering, ON

Client : UPRC c/o Turner & Townsend



END OF BOREHOLE

Dry and open upon completion of drilling.
 50 mm dia. monitoring well installed.
 No. 10 screen

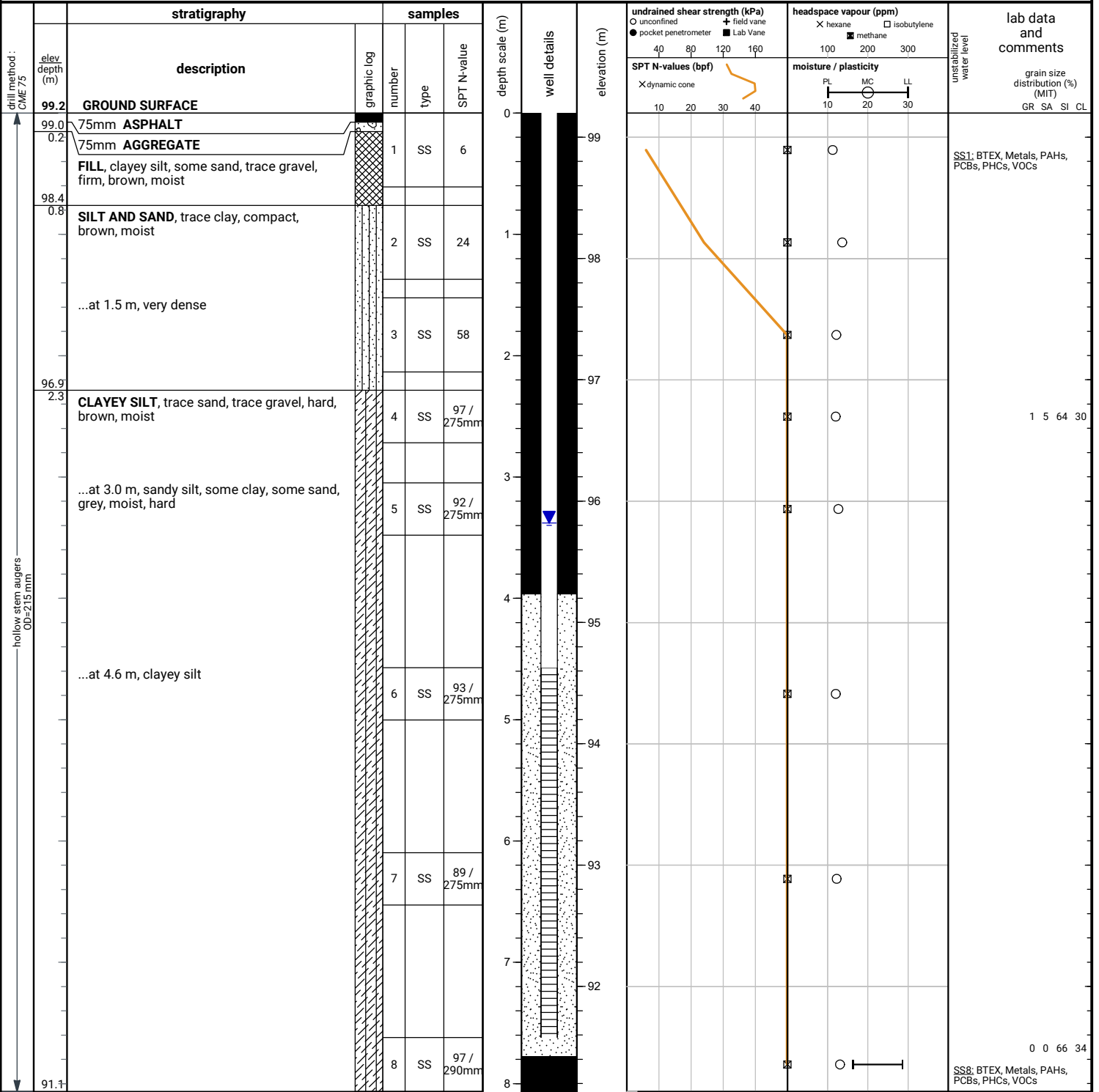
GROUNDWATER LEVELS

date	depth (m)	elevation (m)
May 12, 2022	1.6	98.7
May 27, 2022	1.6	98.7
Jun 22, 2022	1.7	98.6

File No. : 22-088

Project : 1066 Dunbarton Rd., Pickering, ON

Client : UPRC c/o Turner & Townsend



END OF BOREHOLE

Dry and open upon completion of drilling.
 50 mm dia. monitoring well installed.
 No. 10 screen

GROUNDWATER LEVELS

date	depth (m)	elevation (m)
May 12, 2022	5.4	93.8
May 26, 2022	3.7	95.5
Jun 22, 2022	3.4	95.8

APPENDIX C





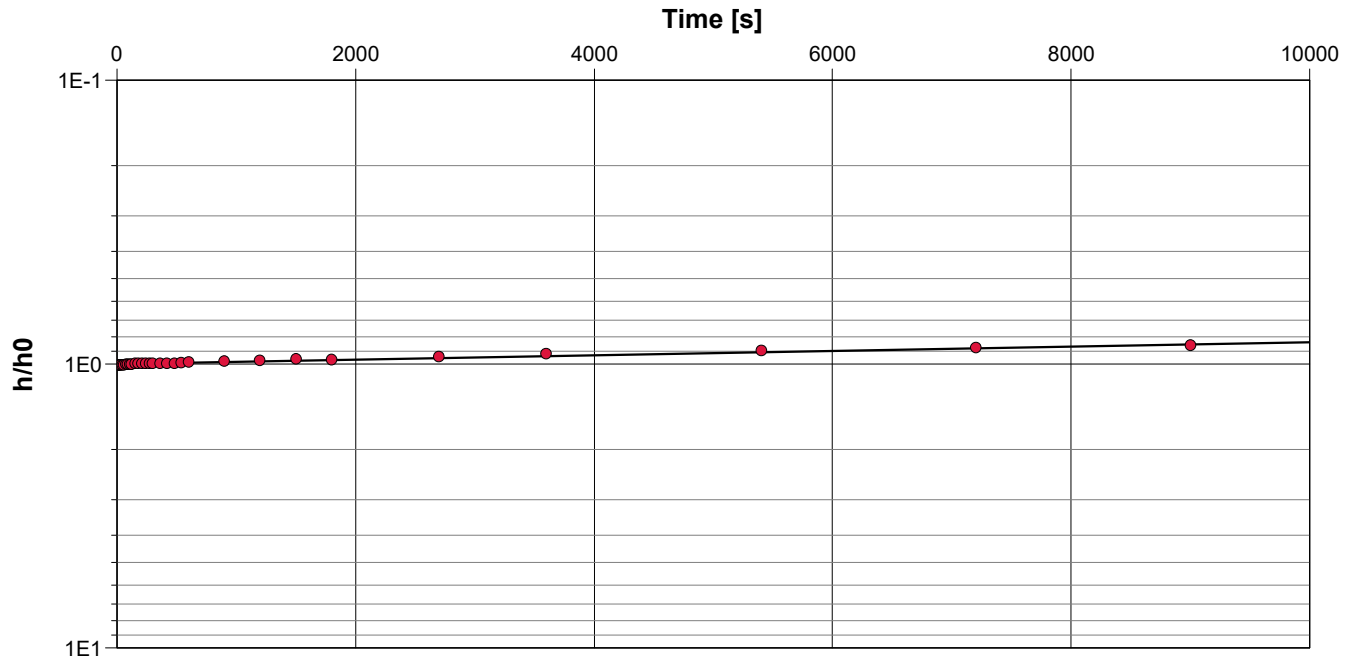
Slug Test Analysis Report

Project: 1066 Dunbarton Rd., Pickering

Number: 22-088

Client: Turner & Townsend

Location: 1066 Dunbarton Rd., Pickering	Slug Test: BH2	Test Well: BH2
Test Conducted by: LB		Test Date: 2022-05-31
Analysis Performed by: SB	Bouwer & Rice	Analysis Date: 2022-05-31
Aquifer Thickness: 7.92 m		



Calculation using Bouwer & Rice

Observation Well	Hydraulic Conductivity [m/s]	
BH2	7.21×10^{-9}	



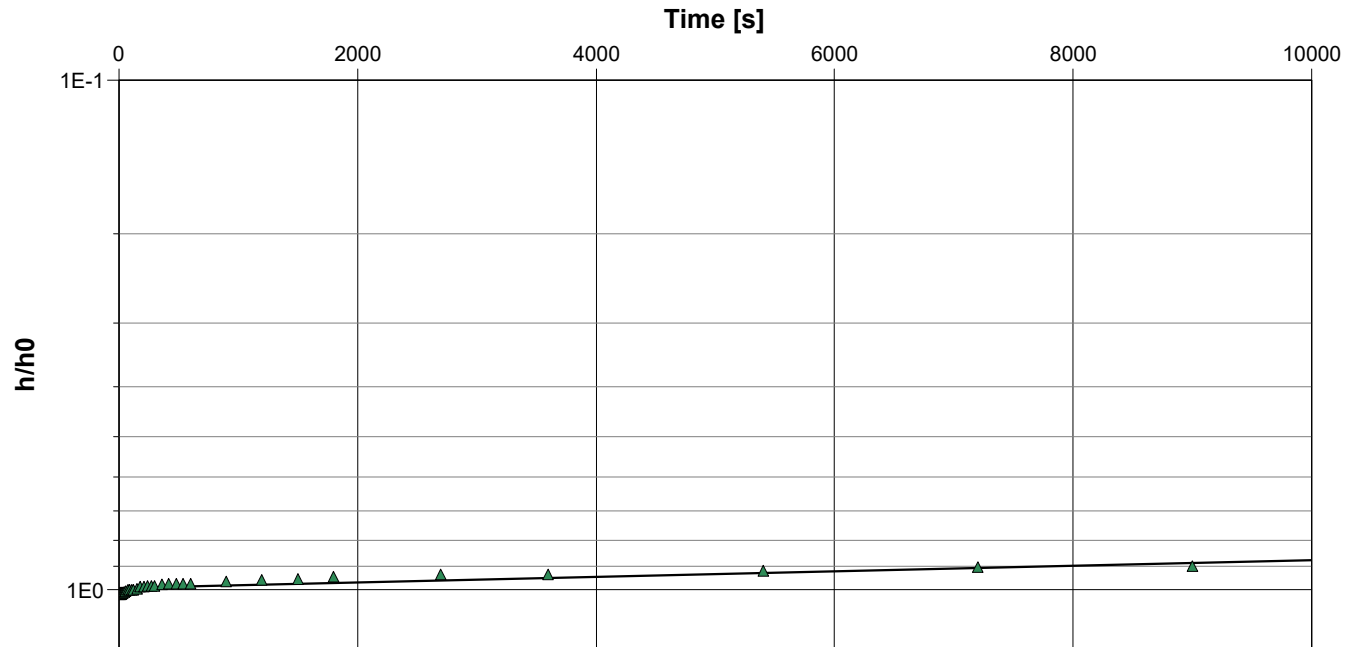
Slug Test Analysis Report

Project: 1066 Dunbarton Rd., Pickering

Number: 22-088

Client: Turner & Townsend

Location: 1066 Dunbarton Rd., Pickering	Slug Test: BH3	Test Well: BH3
Test Conducted by: LB		Test Date: 2022-05-12
Analysis Performed by: SB	Bouwer & Rice	Analysis Date: 2022-05-31
Aquifer Thickness: 7.92 m		

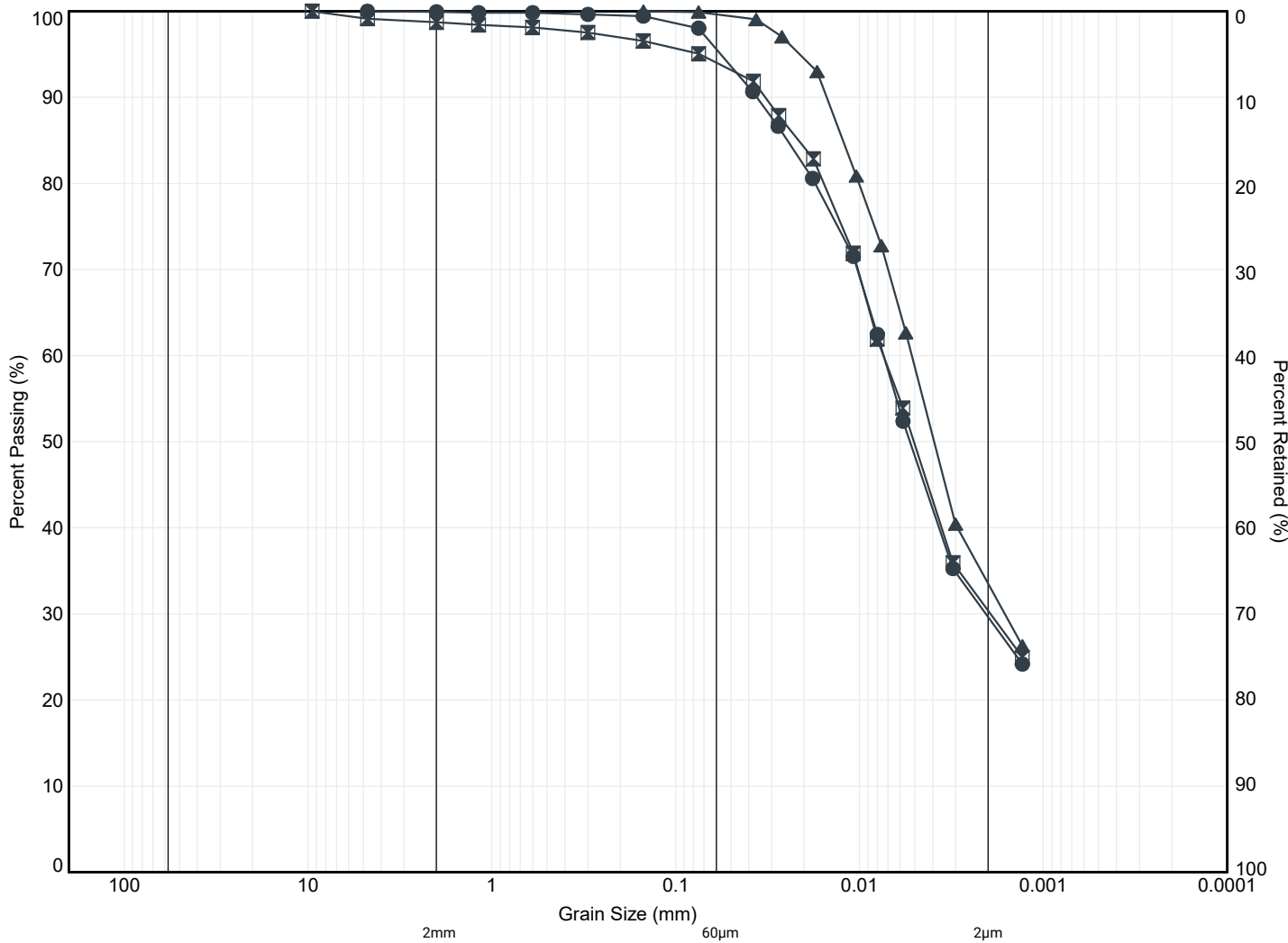


Calculation using Bouwer & Rice

Observation Well	Hydraulic Conductivity [m/s]	
BH3	5.10×10^{-9}	

APPENDIX D





MIT SYSTEM	COBBLES	GRAVEL			SAND			SILT	CLAY
		COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE		

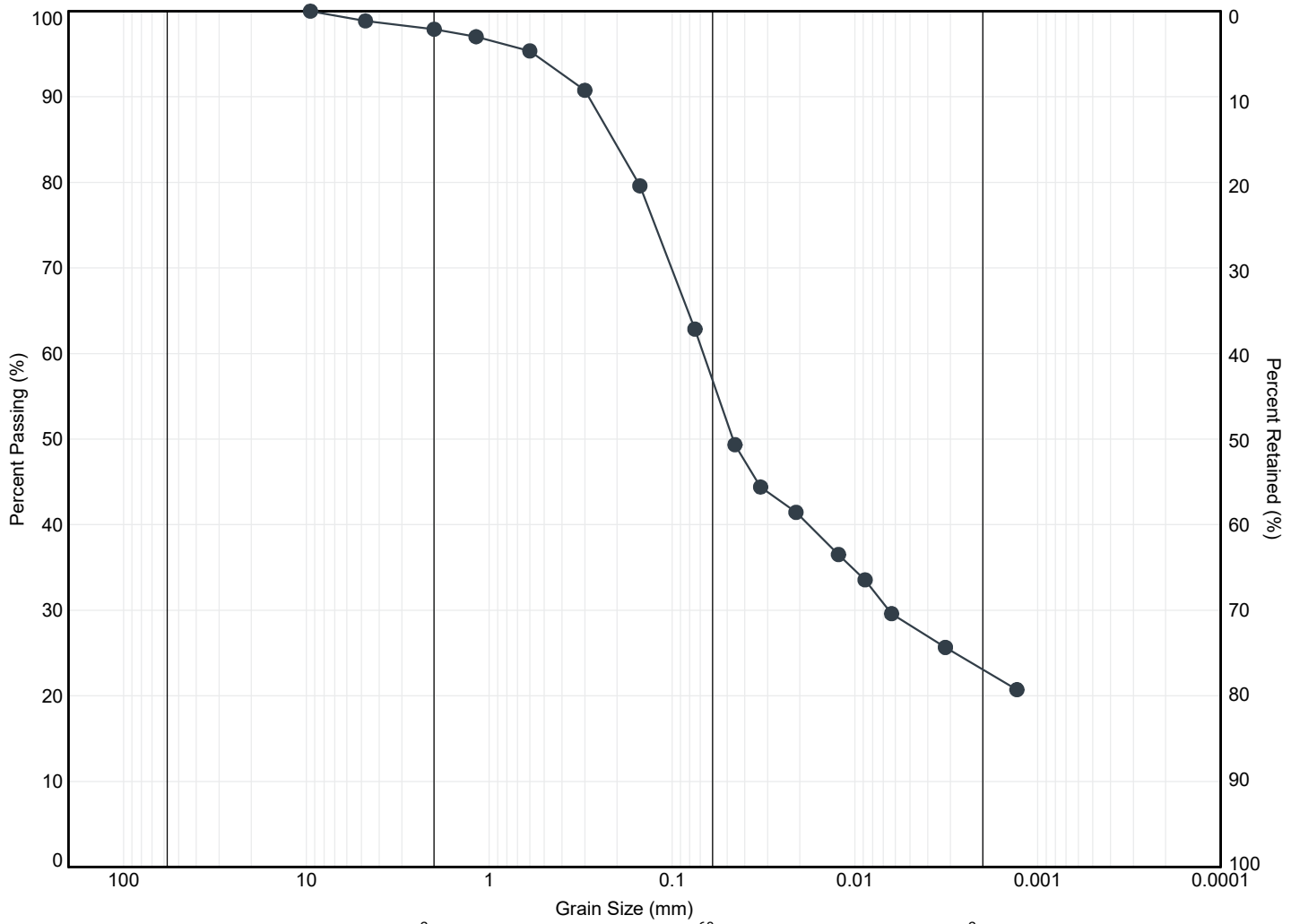
MIT SYSTEM

	Borehole	Sample	Depth (m)	Elev. (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)
●	2	SS6	4.7	95.6	0	4	66	30
☒	3	SS4	2.5	96.7	1	5	64	30
▲	3	SS8	7.8	91.4	0	0	66	34

Title:	GRAIN SIZE DISTRIBUTION
File No.:	22-088

file: 22-088.grit.gpi





MIT SYSTEM	COBBLES	GRAVEL			SAND			SILT	CLAY
		COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE		

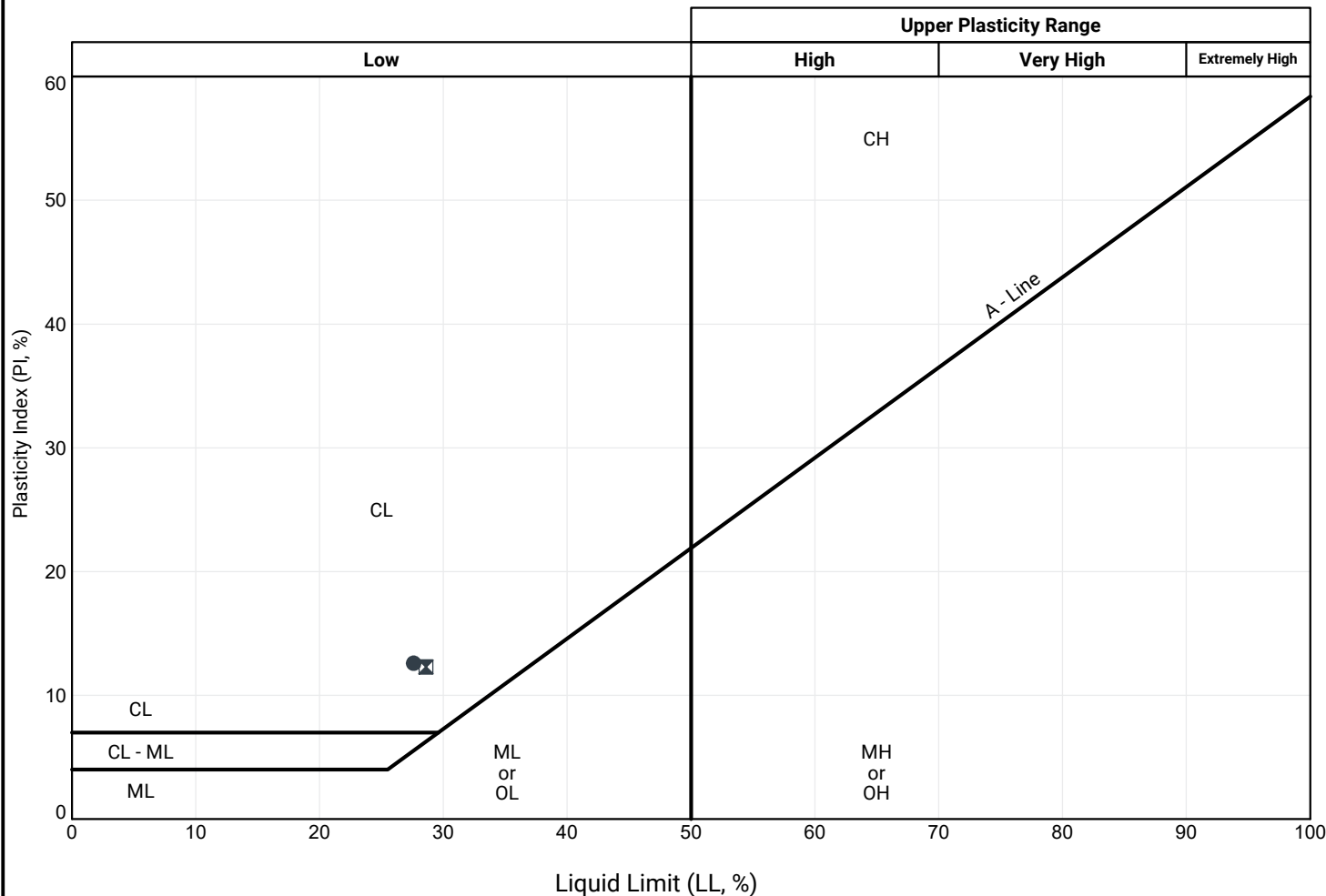
MIT SYSTEM								
Borehole	Sample	Depth (m)	Elev. (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	
● 1	SS3	1.8	96.9	2	41	34	23	

file: 22-088.gint.gpj



Title: **GRAIN SIZE DISTRIBUTION**

File No.: **22-088**



Borehole	Sample	Depth (m)	Elev. (m)	LL (%)	PL (%)	PI (%)
● 2	SS6	4.7	95.6	28	15	13
⊠ 3	SS8	7.8	91.4	29	16	13

APPENDIX E





K from Grain Size Analysis Report

Date: 10-Jun-22

Sample Name:

BH1 SS3

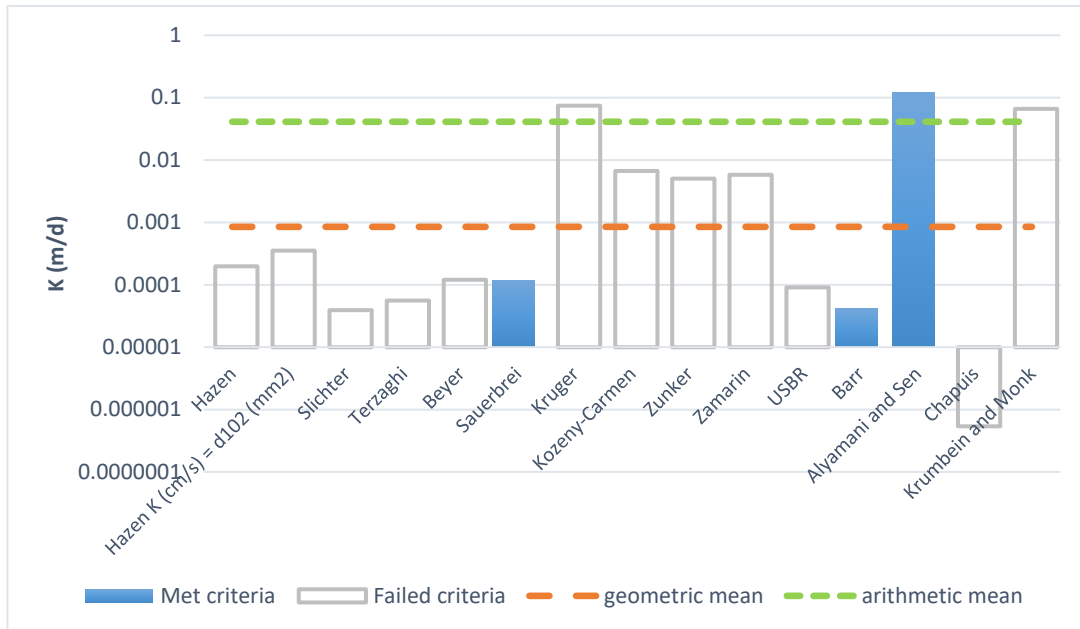
Mass Sample (g):

152.8

T (oC)

20

Poorly sorted sandy silt with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	2.3E-07	2.3E-09	0.00	
Hazen K (cm/s) = d ₁₀ (mm)	4.1E-07	4.1E-09	0.00	
Slichter	4.5E-08	4.5E-10	0.00	
Terzaghi	6.4E-08	6.4E-10	0.00	
Beyer	1.4E-07	1.4E-09	0.00	
Sauerbrei	1.4E-07	1.4E-09	0.00	
Kruger	8.6E-05	8.6E-07	0.07	
Kozeny-Carmen	7.7E-06	7.7E-08	0.01	
Zunker	5.8E-06	5.8E-08	0.01	
Zamarin	6.7E-06	6.7E-08	0.01	
USBR	1.0E-07	1.0E-09	0.00	
Barr	4.8E-08	4.8E-10	0.00	
Alyamani and Sen	1.4E-04	1.4E-06	0.12	
Chapuis	6.2E-10	6.2E-12	0.00	
Krumbein and Monk	7.7E-05	7.7E-07	0.07	
geometric mean	9.8E-07	9.8E-09	0.00	
arithmetic mean	4.8E-05	4.8E-07	0.04	



K from Grain Size Analysis Report

Date: 13-May-22

Sample Name:

BH2 SS6

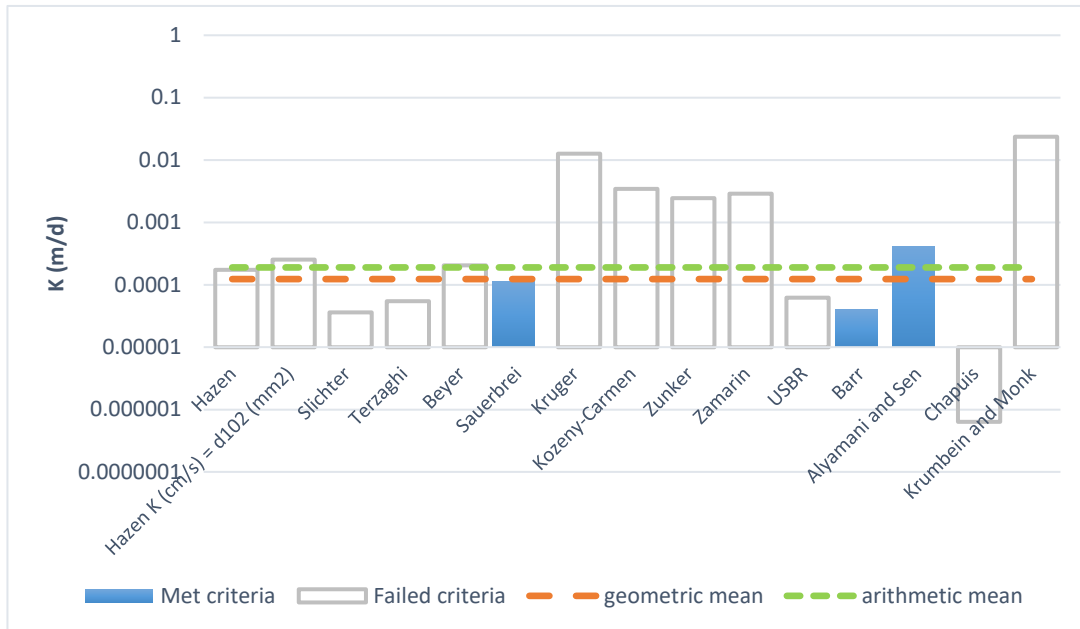
Mass Sample (g):

198.47

T (oC)

20

Poorly sorted clay with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	2.0E-07	2.0E-09	0.00	
Hazen K (cm/s) = d ₁₀ (mm)	2.9E-07	2.9E-09	0.00	
Slichter	4.2E-08	4.2E-10	0.00	
Terzaghi	6.3E-08	6.3E-10	0.00	
Beyer	2.4E-07	2.4E-09	0.00	
Sauerbrei	1.3E-07	1.3E-09	0.00	
Kruger	1.5E-05	1.5E-07	0.01	
Kozeny-Carmen	4.0E-06	4.0E-08	0.00	
Zunker	2.8E-06	2.8E-08	0.00	
Zamarin	3.4E-06	3.4E-08	0.00	
USBR	7.2E-08	7.2E-10	0.00	
Barr	4.6E-08	4.6E-10	0.00	
Alyamani and Sen	4.8E-07	4.8E-09	0.00	
Chapuis	7.4E-10	7.4E-12	0.00	
Krumbein and Monk	2.7E-05	2.7E-07	0.02	
geometric mean	1.4E-07	1.4E-09	0.00	
arithmetic mean	2.2E-07	2.2E-09	0.00	



K from Grain Size Analysis Report

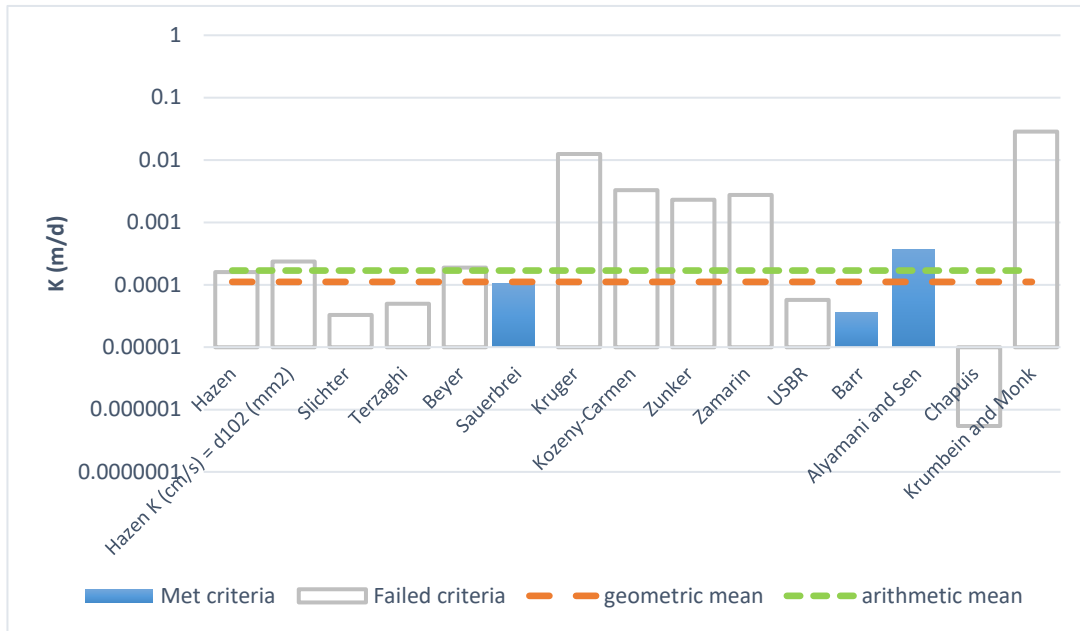
Date: 13-May-22

Sample Name: BH3 SS4

Mass Sample (g): 167.5

T (oC) 20

Poorly sorted clay with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	1.8E-07	1.8E-09	0.00	
Hazen K (cm/s) = d ₁₀ (mm)	2.7E-07	2.7E-09	0.00	
Slichter	3.8E-08	3.8E-10	0.00	
Terzaghi	5.8E-08	5.8E-10	0.00	
Beyer	2.2E-07	2.2E-09	0.00	
Sauerbrei	1.2E-07	1.2E-09	0.00	
Kruger	1.4E-05	1.4E-07	0.01	
Kozeny-Carmen	3.8E-06	3.8E-08	0.00	
Zunker	2.7E-06	2.7E-08	0.00	
Zamarin	3.2E-06	3.2E-08	0.00	
USBR	6.7E-08	6.7E-10	0.00	
Barr	4.2E-08	4.2E-10	0.00	
Alyamani and Sen	4.3E-07	4.3E-09	0.00	
Chapuis	6.3E-10	6.3E-12	0.00	
Krumbein and Monk	3.3E-05	3.3E-07	0.03	
geometric mean	1.3E-07	1.3E-09	0.00	
arithmetic mean	2.0E-07	2.0E-09	0.00	



K from Grain Size Analysis Report

Date: 13-May-22

Sample Name:

BH3 SS8

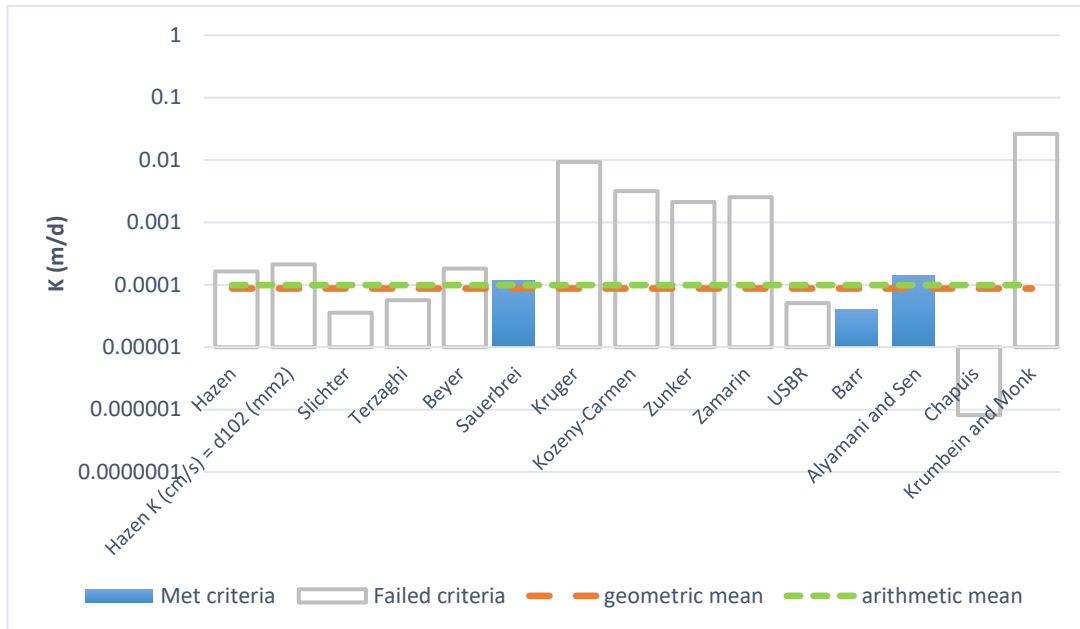
Mass Sample (g):

213.32

T (oC)

20

Poorly sorted clay with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	1.9E-07	1.9E-09	0.00	
Hazen K (cm/s) = d ₁₀ (mm)	2.5E-07	2.5E-09	0.00	
Slichter	4.1E-08	4.1E-10	0.00	
Terzaghi	6.5E-08	6.5E-10	0.00	
Beyer	2.1E-07	2.1E-09	0.00	
Sauerbrei	1.3E-07	1.3E-09	0.00	
Kruger	1.1E-05	1.1E-07	0.01	
Kozeny-Carmen	3.7E-06	3.7E-08	0.00	
Zunker	2.5E-06	2.5E-08	0.00	
Zamarin	2.9E-06	2.9E-08	0.00	
USBR	5.9E-08	5.9E-10	0.00	
Barr	4.7E-08	4.7E-10	0.00	
Alyamani and Sen	1.6E-07	1.6E-09	0.00	
Chapuis	9.4E-10	9.4E-12	0.00	
Krumbein and Monk	3.0E-05	3.0E-07	0.03	
geometric mean	1.0E-07	1.0E-09	0.00	
arithmetic mean	1.1E-07	1.1E-09	0.00	

APPENDIX F





Grounded Engineering Inc
ATTN: Sam Baston
1 Banigan Drive
TORONTO ON M4H 1G3

Date Received: 12-MAY-22
Report Date: 25-MAY-22 08:58 (MT)
Version: FINAL

Client Phone: 647-361-5136

Certificate of Analysis

Lab Work Order #: L2706126
Project P.O. #: NOT SUBMITTED
Job Reference: 22-088
C of C Numbers: 20-951299
Legal Site Desc:

Amanda Overholster
Account Manager

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Summary of Guideline Exceedances

Guideline		Grouping	Analyte	Result	Guideline Limit	Unit
ALS ID	Client ID					
Ontario Sewer Use Bylaws - Durham Sanitary Sewer (55-2013)						
(No parameter exceedances)						
Ontario Sewer Use Bylaws - Durham Storm Sewer - (55-2013)						
L2706126-1	BH3	Physical Tests	Total Suspended Solids	110	15	mg/L

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Physical Tests - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
pH	pH units	6.00-10.5	6.0-9.0	8.37 ^{PEHT}
Total Suspended Solids	mg/L	350	15	110

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Anions and Nutrients - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Fluoride (F)	mg/L	10	-	0.479
Total Kjeldahl Nitrogen	mg/L	100	1	0.414
Phosphorus, Total	mg/L	10	0.4	0.0349
Sulfate (SO4)	mg/L	1500	-	39.7

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

- Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.
- Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Cyanides - WATER


Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3


Guide Limits
Unit #1 #2

Analyte	Unit	#1	#2	
Cyanide, Total	mg/L	2	0.02	<0.0020

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

 Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

 Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

Bacteriological Tests - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Guide Limits
Unit #1 #2

Analyte	Unit	#1	#2
E. Coli	CFU/100m L	-	200
			0

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

- Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.
- Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Total Metals - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Aluminum (Al)-Total	mg/L	50	-	0.296
Antimony (Sb)-Total	mg/L	5	-	0.00071
Arsenic (As)-Total	mg/L	1	0.02	0.00310
Cadmium (Cd)-Total	mg/L	0.7	0.008	<0.000010
Chromium (Cr)-Total	mg/L	2	0.08	0.00067
Cobalt (Co)-Total	mg/L	5	-	0.00032
Copper (Cu)-Total	mg/L	3	0.05	0.0018
Lead (Pb)-Total	mg/L	1	0.12	0.00029
Manganese (Mn)-Total	mg/L	5	0.15	0.0217
Mercury (Hg)-Total	mg/L	0.01	0.0004	<0.0000050
Molybdenum (Mo)-Total	mg/L	5	-	0.0298
Nickel (Ni)-Total	mg/L	2	0.08	0.00103
Selenium (Se)-Total	mg/L	1	0.02	0.000187
Silver (Ag)-Total	mg/L	5	0.12	0.000089
Tin (Sn)-Total	mg/L	5	-	0.00387
Titanium (Ti)-Total	mg/L	5	-	0.00774
Zinc (Zn)-Total	mg/L	2	0.04	<0.0030

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Aggregate Organics - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Guide Limits
Unit #1 #2

Analyte	Unit	#1	#2	
BOD	mg/L	300	15	<3.0 ^{BODL}
Oil and Grease, Total	mg/L	-	-	<5.0
Animal/Veg Oil & Grease	mg/L	150	-	<5.0
Mineral Oil and Grease	mg/L	15	-	<2.5
Phenols (4AAP)	mg/L	1	0.008	<0.0010

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

- Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.
- Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

Volatile Organic Compounds - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Benzene	ug/L	10	2	<0.50
Chloroform	ug/L	40	2	<1.0
1,2-Dichlorobenzene	ug/L	50	5.6	<0.50
1,4-Dichlorobenzene	ug/L	80	6.8	<0.50
cis-1,2-Dichloroethylene	ug/L	4000	5.6	<0.50
Dichloromethane	ug/L	2000	5.2	<2.0
trans-1,3-Dichloropropene	ug/L	140	5.6	<0.50
Ethylbenzene	ug/L	160	2	<0.50
Methyl Ethyl Ketone	ug/L	8000	-	<20
Styrene	ug/L	200	-	<0.50
1,1,1,2-Tetrachloroethane	ug/L	1400	17	<0.50
Tetrachloroethylene	ug/L	1000	4.4	<0.50
Toluene	ug/L	270	2	<0.50
Trichloroethylene	ug/L	400	8	<0.50
o-Xylene	ug/L	-	-	<0.50
m+p-Xylenes	ug/L	-	-	<1.0
Xylenes (Total)	ug/L	1400	4.4	<1.1
Surrogate: 4-Bromofluorobenzene	%	-	-	94.4
Surrogate: 1,4-Difluorobenzene	%	-	-	101.2

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.
 Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Phthalate Esters - WATER


Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3


Guide Limits
#1 #2

Analyte	Unit	#1	#2	
Bis(2-ethylhexyl)phthalate	ug/L	12	8.8	<2.0
Surrogate: 2-fluorobiphenyl	%	-	-	87.8
Surrogate: p-Terphenyl d14	%	-	-	102.2

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

 Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

 Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.


Semi-Volatile Organics - WATER


Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Di-n-butylphthalate	ug/L	80	15	1.7
Surrogate: 2-Fluorobiphenyl	%	-	-	87.8
Surrogate: p-Terphenyl d14	%	-	-	102.2

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

 Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

 Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

Polychlorinated Biphenyls - WATER

Lab ID L2706126-1
Sample Date 12-MAY-22
Sample ID BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Aroclor 1242	ug/L	-	-	<0.020
Aroclor 1248	ug/L	-	-	<0.020
Aroclor 1254	ug/L	-	-	<0.020
Aroclor 1260	ug/L	-	-	<0.020
Surrogate: Decachlorobiphenyl	%	-	-	142.7
Total PCBs	ug/L	1	0.4	<0.040
Surrogate: Tetrachloro-m-xylene	%	-	-	99.2

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.

Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Organic Parameters - WATER

Lab ID	L2706126-1
Sample Date	12-MAY-22
Sample ID	BH3

Analyte	Unit	Guide Limits		
		#1	#2	
Nonylphenol	ug/L	20	-	10.9
Nonylphenol Diethoxylates	ug/L	-	-	<0.10
Total Nonylphenol Ethoxylates	ug/L	200	-	<2.0
Nonylphenol Monoethoxylates	ug/L	-	-	<2.0

Guide Limit #1: Durham Sanitary Sewer (55-2013)

Guide Limit #2: Durham Storm Sewer - (55-2013)

- Detection Limit for result exceeds Guideline Limit. Assessment against Guideline Limit cannot be made.
- Analytical result for this parameter exceeds Guide Limits listed. See Summary of Guideline Exceedances.

* Please refer to the Reference Information section for an explanation of any qualifiers noted.

Reference Information

Qualifiers for Sample Submission Listed:

Qualifier	Description
CINT	Cooling initiated. Samples were received packed with ice or ice packs and were sampled the same day as received.

Qualifiers for Individual Parameters Listed:

Qualifier	Description
PEHT	Parameter Exceeded Recommended Holding Time Prior to Analysis
BODL	Limit of Reporting for BOD was increased to account for the largest volume of sample tested.

Methods Listed (if applicable):

ALS Test Code	Matrix	Test Description	Method Reference**
625-BIS-2-PHTH-WT	Water	Bis(2-ethylhexyl)phthalate	SW846 8270
Aqueous samples are extracted and extracts are analyzed on GC/MSD.			
625-DNB-PHTH-WT	Water	Di-n-Butyl Phthalate	SW846 8270
Aqueous samples are extracted and extracts are analyzed on GC/MSD.			
BOD-WT	Water	BOD	APHA 5210 B
This analysis is carried out using procedures adapted from APHA Method 5210B - "Biochemical Oxygen Demand (BOD)". All forms of biochemical oxygen demand (BOD) are determined by diluting and incubating a sample for a specified time period, and measuring the oxygen depletion using a dissolved oxygen meter. Dissolved BOD (SOLUBLE) is determined by filtering the sample through a glass fibre filter prior to dilution. Carbonaceous BOD (CBOD) is determined by adding a nitrification inhibitor to the diluted sample prior to incubation.			
CN-TOT-WT	Water	Cyanide, Total	ISO 14403-2
Total cyanide is determined by the combination of UV digestion and distillation. Cyanide is converted to cyanogen chloride by reacting with chloramine-T, the cyanogen chloride then reacts with a combination of barbituric acid and isonicotinic acid to form a highly colored complex.			
When using this method, high levels of thiocyanate in samples can cause false positives at ~1-2% of the thiocyanate concentration. For samples with detectable cyanide analyzed by this method, ALS recommends analysis for thiocyanate to check for this potential interference			
EC-SCREEN-WT	Water	Conductivity Screen (Internal Use Only)	APHA 2510
Qualitative analysis of conductivity where required during preparation of other tests - e.g. TDS, metals, etc.			
EC-WW-MF-WT	Water	E. Coli	SM 9222D
A 100 mL volume of sample is filtered through a membrane, the membrane is placed on mFC-BCIG agar and incubated at 44.5 – 0.2 °C for 24 – 2 h. Method ID: WT-TM-1200			
F-IC-N-WT	Water	Fluoride in Water by IC	EPA 300.1 (mod)
Inorganic anions are analyzed by Ion Chromatography with conductivity and/or UV detection.			
HG-T-CVAA-WT	Water	Total Mercury in Water by CVAAS	EPA 1631E (mod)
Water samples undergo a cold-oxidation using bromine monochloride prior to reduction with stannous chloride, and analyzed by CVAAS.			
MET-T-CCMS-WT	Water	Total Metals in Water by CRC ICPMS	EPA 200.2/6020A (mod)
Water samples are digested with nitric and hydrochloric acids, and analyzed by CRC ICPMS.			

Reference Information

Methods Listed (if applicable):

ALS Test Code	Matrix	Test Description	Method Reference**
Method Limitation (re: Sulfur): Sulfide and volatile sulfur species may not be recovered by this method.			
Analysis conducted in accordance with the Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act (July 1, 2011).			
NP,NPE-LCMS-WT	Water	Nonylphenols and Ethoxylates by LC/MS-MS	J. Chrom A849 (1999) p.467-482
Water samples are filtered and analyzed on LCMS/MS by direct injection.			
OGG-SPEC-CALC-WT	Water	Speciated Oil and Grease A/V Calc	CALCULATION
Sample is extracted with hexane, sample speciation into mineral and animal/vegetable fractions is achieved via silica gel separation and is then determined gravimetrically.			
OGG-SPEC-WT	Water	Speciated Oil and Grease-Gravimetric	APHA 5520 B
The procedure involves an extraction of the entire water sample with hexane. Sample speciation into mineral and animal/vegetable fractions is achieved via silica gel separation and is then determined gravimetrically.			
P-T-COL-WT	Water	Total P in Water by Colour	APHA 4500-P PHOSPHORUS
This analysis is carried out using procedures adapted from APHA Method 4500-P "Phosphorus". Total Phosphorus is determined colourimetrically after persulphate digestion of the sample.			
PCB-WT	Water	Polychlorinated Biphenyls	EPA 8082
PCBs are extracted from an aqueous sample at neutral pH with aliquots of dichloromethane using a modified separatory funnel technique. The extracts are analyzed by GC/MSD.			
PH-WT	Water	pH	APHA 4500 H-Electrode
Water samples are analyzed directly by a calibrated pH meter.			
Analysis conducted in accordance with the Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act (July 1, 2011). Holdtime for samples under this regulation is 28 days			
PHENOLS-4AAP-WT	Water	Phenol (4AAP)	EPA 9066
An automated method is used to distill the sample. The distillate is then buffered to pH 9.4 which reacts with 4AAP and potassium ferricyanide to form a red complex which is measured colorimetrically.			
SO4-IC-N-WT	Water	Sulfate in Water by IC	EPA 300.1 (mod)
Inorganic anions are analyzed by Ion Chromatography with conductivity and/or UV detection.			
SOLIDS-TSS-WT	Water	Suspended solids	APHA 2540 D-Gravimetric
A well-mixed sample is filtered through a weighed standard glass fibre filter and the residue retained is dried in an oven at 104–1°C for a minimum of four hours or until a constant weight is achieved.			
TKN-F-WT	Water	TKN in Water by Fluorescence	J. ENVIRON. MONIT., 2005,7,37-42,RSC
Total Kjeldahl Nitrogen is determined using block digestion followed by Flow-injection analysis with fluorescence detection			
VOC-ROU-HS-WT	Water	Volatile Organic Compounds	SW846 8260
Aqueous samples are analyzed by headspace-GC/MS.			

Reference Information

Methods Listed (if applicable):

ALS Test Code	Matrix	Test Description	Method Reference**
XYLENES-SUM-CALC-WT	Water	Sum of Xylene Isomer Concentrations	CALCULATION

Total xylenes represents the sum of o-xylene and m&p-xylene.

**ALS test methods may incorporate modifications from specified reference methods to improve performance.

Chain of Custody Numbers:

20-951299

The last two letters of the above test code(s) indicate the laboratory that performed analytical analysis for that test. Refer to the list below:

Laboratory Definition Code	Laboratory Location
WT	ALS ENVIRONMENTAL - WATERLOO, ONTARIO, CANADA

GLOSSARY OF REPORT TERMS

Surrogates are compounds that are similar in behaviour to target analyte(s), but that do not normally occur in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery. In reports that display the D.L. column, laboratory objectives for surrogates are listed there.

mg/kg - milligrams per kilogram based on dry weight of sample

mg/kg wwt - milligrams per kilogram based on wet weight of sample

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight

mg/L - unit of concentration based on volume, parts per million.

< - Less than.

D.L. - The reporting limit.

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, fitness for a particular purpose, or non-infringement. ALS assumes no responsibility for errors or omissions in the information. Guideline limits are not adjusted for the hardness, pH or temperature of the sample (the most conservative values are used). Measurement uncertainty is not applied to test results prior to comparison with specified criteria values.



Quality Control Report

Workorder: L2706126

Report Date: 25-MAY-22

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Client: Grounded Engineering Inc
1 Banigan Drive
TORONTO ON M4H 1G3

Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
625-BIS-2-PHTH-WT Water								
Batch	R5785171							
WG3727801-2	LCS							
Bis(2-ethylhexyl)phthalate			119.3		%		50-140	19-MAY-22
WG3727801-1	MB							
Bis(2-ethylhexyl)phthalate			<2.0		ug/L		2	19-MAY-22
Surrogate: 2-fluorobiphenyl			93.9		%		40-130	19-MAY-22
Surrogate: p-Terphenyl d14			107.0		%		40-130	19-MAY-22
625-DNB-PHTH-WT Water								
Batch	R5785171							
WG3727801-2	LCS							
Di-n-butylphthalate			106.2		%		50-150	19-MAY-22
WG3727801-1	MB							
Di-n-butylphthalate			<1.0		ug/L		1	19-MAY-22
Surrogate: 2-Fluorobiphenyl			93.9		%		40-130	19-MAY-22
Surrogate: p-Terphenyl d14			107.0		%		40-130	19-MAY-22
BOD-WT Water								
Batch	R5785270							
WG3727454-2	DUP	L2706137-1						
BOD		<3.0	<3.0	RPD-NA	mg/L	N/A	30	14-MAY-22
WG3727454-3	LCS							
BOD			91.9		%		85-115	14-MAY-22
WG3727454-1	MB							
BOD			<2.0		mg/L		2	14-MAY-22
CN-TOT-WT Water								
Batch	R5784019							
WG3728528-3	DUP	WG3728528-5						
Cyanide, Total		<0.0020	<0.0020	RPD-NA	mg/L	N/A	20	17-MAY-22
WG3728528-2	LCS							
Cyanide, Total			105.1		%		80-120	17-MAY-22
WG3728528-1	MB							
Cyanide, Total			<0.0020		mg/L		0.002	17-MAY-22
WG3728528-4	MS	WG3728528-5						
Cyanide, Total			89.0		%		70-130	17-MAY-22
EC-WW-MF-WT Water								
Batch	R5780478							
WG3727377-1	MB							
E. Coli			0		CFU/100mL		1	14-MAY-22



Quality Control Report

Workorder: L2706126

Report Date: 25-MAY-22

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Client: Grounded Engineering Inc
1 Banigan Drive
TORONTO ON M4H 1G3

Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
F-IC-N-WT		Water						
Batch	R5784093							
WG3728239-4	DUP	WG3728239-3						
Fluoride (F)		0.073	0.073		mg/L	0.1	20	17-MAY-22
WG3728239-2	LCS							
Fluoride (F)			102.0		%		90-110	17-MAY-22
WG3728239-1	MB							
Fluoride (F)			<0.020		mg/L		0.02	17-MAY-22
WG3728239-5	MS	WG3728239-3						
Fluoride (F)			95.3		%		75-125	17-MAY-22
HG-T-CVAA-WT		Water						
Batch	R5781344							
WG3727585-3	DUP	WG3727585-5						
Mercury (Hg)-Total		<0.0000050	<0.0000050	RPD-NA	mg/L	N/A	20	16-MAY-22
WG3727585-2	LCS							
Mercury (Hg)-Total			99.2		%		80-120	16-MAY-22
WG3727585-1	MB							
Mercury (Hg)-Total			<0.0000050		mg/L		0.000005	16-MAY-22
WG3727585-4	MS	WG3727585-6						
Mercury (Hg)-Total			91.3		%		70-130	16-MAY-22
MET-T-CCMS-WT		Water						
Batch	R5781260							
WG3727506-4	DUP	WG3727506-3						
Aluminum (Al)-Total		<0.050	<0.050	RPD-NA	mg/L	N/A	20	16-MAY-22
Antimony (Sb)-Total		<0.0010	<0.0010	RPD-NA	mg/L	N/A	20	16-MAY-22
Arsenic (As)-Total		<0.0010	<0.0010	RPD-NA	mg/L	N/A	20	16-MAY-22
Cadmium (Cd)-Total		<0.000050	<0.000050	RPD-NA	mg/L	N/A	20	16-MAY-22
Chromium (Cr)-Total		<0.0050	<0.0050	RPD-NA	mg/L	N/A	20	16-MAY-22
Cobalt (Co)-Total		<0.0010	<0.0010	RPD-NA	mg/L	N/A	20	16-MAY-22
Copper (Cu)-Total		<0.0050	<0.0050	RPD-NA	mg/L	N/A	20	16-MAY-22
Lead (Pb)-Total		<0.00050	<0.00050	RPD-NA	mg/L	N/A	20	16-MAY-22
Manganese (Mn)-Total		0.105	0.105		mg/L	0.2	20	16-MAY-22
Molybdenum (Mo)-Total		<0.00050	<0.00050	RPD-NA	mg/L	N/A	20	16-MAY-22
Nickel (Ni)-Total		<0.0050	<0.0050	RPD-NA	mg/L	N/A	20	16-MAY-22
Selenium (Se)-Total		<0.00050	<0.00050	RPD-NA	mg/L	N/A	20	16-MAY-22
Silver (Ag)-Total		<0.00050	<0.00050	RPD-NA	mg/L	N/A	20	16-MAY-22
Tin (Sn)-Total		<0.0010	<0.0010	RPD-NA	mg/L	N/A	20	16-MAY-22
Titanium (Ti)-Total		<0.0030	<0.0030	RPD-NA	mg/L	N/A	20	16-MAY-22



Quality Control Report

Workorder: L2706126

Report Date: 25-MAY-22

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Client: Grounded Engineering Inc
1 Banigan Drive
TORONTO ON M4H 1G3

Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
MET-T-CCMS-WT								
	Water							
Batch	R5781260							
WG3727506-4	DUP	WG3727506-3						
Zinc (Zn)-Total		<0.030	<0.030	RPD-NA	mg/L	N/A	20	16-MAY-22
WG3727506-2	LCS							
Aluminum (Al)-Total			92.1		%		80-120	16-MAY-22
Antimony (Sb)-Total			98.5		%		80-120	16-MAY-22
Arsenic (As)-Total			95.0		%		80-120	16-MAY-22
Cadmium (Cd)-Total			95.1		%		80-120	16-MAY-22
Chromium (Cr)-Total			92.8		%		80-120	16-MAY-22
Cobalt (Co)-Total			94.5		%		80-120	16-MAY-22
Copper (Cu)-Total			92.6		%		80-120	16-MAY-22
Lead (Pb)-Total			98.7		%		80-120	16-MAY-22
Manganese (Mn)-Total			94.8		%		80-120	16-MAY-22
Molybdenum (Mo)-Total			92.0		%		80-120	16-MAY-22
Nickel (Ni)-Total			94.3		%		80-120	16-MAY-22
Selenium (Se)-Total			94.1		%		80-120	16-MAY-22
Silver (Ag)-Total			89.8		%		80-120	16-MAY-22
Tin (Sn)-Total			95.5		%		80-120	16-MAY-22
Titanium (Ti)-Total			87.4		%		80-120	16-MAY-22
Zinc (Zn)-Total			91.5		%		80-120	16-MAY-22
WG3727506-1	MB							
Aluminum (Al)-Total			<0.0050		mg/L		0.005	16-MAY-22
Antimony (Sb)-Total			<0.00010		mg/L		0.0001	16-MAY-22
Arsenic (As)-Total			<0.00010		mg/L		0.0001	16-MAY-22
Cadmium (Cd)-Total			<0.0000050		mg/L		0.000005	16-MAY-22
Chromium (Cr)-Total			<0.00050		mg/L		0.0005	16-MAY-22
Cobalt (Co)-Total			<0.00010		mg/L		0.0001	16-MAY-22
Copper (Cu)-Total			<0.00050		mg/L		0.0005	16-MAY-22
Lead (Pb)-Total			<0.000050		mg/L		0.00005	16-MAY-22
Manganese (Mn)-Total			<0.00050		mg/L		0.0005	16-MAY-22
Molybdenum (Mo)-Total			<0.000050		mg/L		0.00005	16-MAY-22
Nickel (Ni)-Total			<0.00050		mg/L		0.0005	16-MAY-22
Selenium (Se)-Total			<0.000050		mg/L		0.00005	16-MAY-22
Silver (Ag)-Total			<0.000050		mg/L		0.00005	16-MAY-22
Tin (Sn)-Total			<0.00010		mg/L		0.0001	16-MAY-22
Titanium (Ti)-Total			<0.00030		mg/L		0.0003	16-MAY-22



Quality Control Report

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Client: Grounded Engineering Inc
1 Banigan Drive
TORONTO ON M4H 1G3

Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
MET-T-CCMS-WT		Water						
Batch	R5781260							
WG3727506-1	MB							
Zinc (Zn)-Total			<0.0030		mg/L		0.003	16-MAY-22
WG3727506-5	MS	WG3727506-6						
Aluminum (Al)-Total			95.9		%		70-130	16-MAY-22
Antimony (Sb)-Total			N/A	MS-B	%		-	16-MAY-22
Arsenic (As)-Total			100.2		%		70-130	16-MAY-22
Cadmium (Cd)-Total			99.0		%		70-130	16-MAY-22
Chromium (Cr)-Total			102.1		%		70-130	16-MAY-22
Cobalt (Co)-Total			N/A	MS-B	%		-	16-MAY-22
Copper (Cu)-Total			N/A	MS-B	%		-	16-MAY-22
Lead (Pb)-Total			100.1		%		70-130	16-MAY-22
Manganese (Mn)-Total			N/A	MS-B	%		-	16-MAY-22
Molybdenum (Mo)-Total			N/A	MS-B	%		-	16-MAY-22
Nickel (Ni)-Total			95.1		%		70-130	16-MAY-22
Selenium (Se)-Total			102.5		%		70-130	16-MAY-22
Silver (Ag)-Total			87.0		%		70-130	16-MAY-22
Tin (Sn)-Total			98.2		%		70-130	16-MAY-22
Titanium (Ti)-Total			98.1		%		70-130	16-MAY-22
Zinc (Zn)-Total			97.2		%		70-130	16-MAY-22
NP,NPE-LCMS-WT		Water						
Batch	R5784526							
WG3727734-3	DUP	L2706038-1						
Nonylphenol		<1.0	<1.0	RPD-NA	ug/L	N/A	30	17-MAY-22
Nonylphenol Monoethoxylates		<2.0	<2.0	RPD-NA	ug/L	N/A	30	17-MAY-22
Nonylphenol Diethoxylates		<0.10	<0.10	RPD-NA	ug/L	N/A	30	17-MAY-22
WG3727734-2	LCS							
Nonylphenol			94.8		%		75-125	17-MAY-22
Nonylphenol Monoethoxylates			95.2		%		75-125	17-MAY-22
Nonylphenol Diethoxylates			99.5		%		75-125	17-MAY-22
WG3727734-1	MB							
Nonylphenol			<1.0		ug/L		1	17-MAY-22
Nonylphenol Monoethoxylates			<2.0		ug/L		2	17-MAY-22
Nonylphenol Diethoxylates			<0.10		ug/L		0.1	17-MAY-22
WG3727734-4	MS	L2706038-1						
Nonylphenol			143.4	K	%		60-140	17-MAY-22



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Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
NP,NPE-LCMS-WT								
	Water							
Batch	R5784526							
WG3727734-4 MS		L2706038-1						
Nonylphenol Monoethoxylates			151.4	K	%		60-140	17-MAY-22
Nonylphenol Diethoxylates			85.7		%		60-140	17-MAY-22
OGG-SPEC-WT								
	Water							
Batch	R5786461							
WG3729220-2 LCS								
Oil and Grease, Total			103.3		%		70-130	19-MAY-22
Mineral Oil and Grease			96.5		%		70-130	19-MAY-22
WG3729220-1 MB								
Oil and Grease, Total			<5.0		mg/L		5	19-MAY-22
Mineral Oil and Grease			<2.5		mg/L		2.5	19-MAY-22
P-T-COL-WT								
	Water							
Batch	R5782606							
WG3727658-3 DUP		L2705650-8						
Phosphorus, Total		0.0672	0.0555		mg/L	19	20	17-MAY-22
WG3727658-2 LCS								
Phosphorus, Total			97.3		%		80-120	17-MAY-22
WG3727658-1 MB								
Phosphorus, Total			<0.0030		mg/L		0.003	17-MAY-22
WG3727658-4 MS		L2705650-8						
Phosphorus, Total			73.5		%		70-130	17-MAY-22
PCB-WT								
	Water							
Batch	R5783805							
WG3728580-2 LCS								
Aroclor 1242			95.0		%		65-130	18-MAY-22
Aroclor 1248			99.6		%		65-130	18-MAY-22
Aroclor 1254			91.9		%		65-130	18-MAY-22
Aroclor 1260			93.8		%		65-130	18-MAY-22
WG3728580-1 MB								
Aroclor 1242			<0.020		ug/L		0.02	18-MAY-22
Aroclor 1248			<0.020		ug/L		0.02	18-MAY-22
Aroclor 1254			<0.020		ug/L		0.02	18-MAY-22
Aroclor 1260			<0.020		ug/L		0.02	18-MAY-22
Surrogate: Decachlorobiphenyl			111.9		%		50-150	18-MAY-22
Surrogate: Tetrachloro-m-xylene			97.0		%		50-150	18-MAY-22
PH-WT								
	Water							



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Client: Grounded Engineering Inc
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Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
PH-WT		Water						
Batch	R5783717							
WG3728172-4	DUP	WG3728172-3						
pH		8.37	8.21	J	pH units	0.16	0.2	17-MAY-22
WG3728172-2	LCS							
pH			7.06		pH units		6.9-7.1	17-MAY-22
PHENOLS-4AAP-WT		Water						
Batch	R5782879							
WG3727762-2	LCS							
Phenols (4AAP)			101.5		%		85-115	16-MAY-22
WG3727762-1	MB							
Phenols (4AAP)			<0.0010		mg/L		0.001	16-MAY-22
SO4-IC-N-WT		Water						
Batch	R5784093							
WG3728239-4	DUP	WG3728239-3						
Sulfate (SO4)		55.8	55.8		mg/L	0.0	20	17-MAY-22
WG3728239-2	LCS							
Sulfate (SO4)			102.7		%		90-110	17-MAY-22
WG3728239-1	MB							
Sulfate (SO4)			<0.30		mg/L		0.3	17-MAY-22
WG3728239-5	MS	WG3728239-3						
Sulfate (SO4)			105.6		%		75-125	17-MAY-22
SOLIDS-TSS-WT		Water						
Batch	R5780957							
WG3727479-3	DUP	L2706250-1						
Total Suspended Solids		72.0	73.0		mg/L	1.4	20	16-MAY-22
WG3727479-2	LCS							
Total Suspended Solids			106.7		%		85-115	16-MAY-22
WG3727479-1	MB							
Total Suspended Solids			<3.0		mg/L		3	16-MAY-22
TKN-F-WT		Water						
Batch	R5784167							
WG3727736-3	DUP	WG3727736-5						
Total Kjeldahl Nitrogen		0.796	0.726		mg/L	9.2	20	17-MAY-22
WG3727736-2	LCS							
Total Kjeldahl Nitrogen			101.3		%		75-125	17-MAY-22
WG3727736-1	MB							
Total Kjeldahl Nitrogen			<0.050		mg/L		0.05	17-MAY-22
WG3727736-4	MS	WG3727736-5						



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Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
TKN-F-WT		Water						
Batch	R5784167							
WG3727736-4 MS		WG3727736-5						
Total Kjeldahl Nitrogen			99.2		%		70-130	17-MAY-22
VOC-ROU-HS-WT		Water						
Batch	R5784216							
WG3728330-4 DUP		WG3728330-3						
1,1,2,2-Tetrachloroethane		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
1,2-Dichlorobenzene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
1,4-Dichlorobenzene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
Benzene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
Chloroform		<1.0	<1.0	RPD-NA	ug/L	N/A	30	18-MAY-22
cis-1,2-Dichloroethylene		0.67	0.67		ug/L	0.0	30	18-MAY-22
Dichloromethane		<2.0	<2.0	RPD-NA	ug/L	N/A	30	18-MAY-22
Ethylbenzene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
m+p-Xylenes		<0.40	<0.40	RPD-NA	ug/L	N/A	30	18-MAY-22
Methyl Ethyl Ketone		<20	<20	RPD-NA	ug/L	N/A	30	18-MAY-22
o-Xylene		<0.30	<0.30	RPD-NA	ug/L	N/A	30	18-MAY-22
Styrene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
Tetrachloroethylene		<0.50	<0.50	RPD-NA	ug/L	N/A	30	18-MAY-22
Toluene		<0.40	<0.40	RPD-NA	ug/L	N/A	30	18-MAY-22
trans-1,3-Dichloropropene		<0.50	<0.30	RPD-NA	ug/L	N/A	30	18-MAY-22
Trichloroethylene		4.52	4.61		ug/L	2.0	30	18-MAY-22
WG3728330-1 LCS								
1,1,2,2-Tetrachloroethane			95.1		%		70-130	17-MAY-22
1,2-Dichlorobenzene			105.2		%		70-130	17-MAY-22
1,4-Dichlorobenzene			106.4		%		70-130	17-MAY-22
Benzene			95.2		%		70-130	17-MAY-22
Chloroform			106.5		%		70-130	17-MAY-22
cis-1,2-Dichloroethylene			109.5		%		70-130	17-MAY-22
Dichloromethane			111.4		%		70-130	17-MAY-22
Ethylbenzene			95.0		%		70-130	17-MAY-22
m+p-Xylenes			101.3		%		70-130	17-MAY-22
Methyl Ethyl Ketone			97.0		%		60-140	17-MAY-22
o-Xylene			95.3		%		70-130	17-MAY-22
Styrene			83.7		%		70-130	17-MAY-22



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Client: Grounded Engineering Inc
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Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
VOC-ROU-HS-WT								
	Water							
Batch	R5784216							
WG3728330-1	LCS							
Tetrachloroethylene			95.1		%		70-130	17-MAY-22
Toluene			97.1		%		70-130	17-MAY-22
trans-1,3-Dichloropropene			62.7	MES	%		70-130	17-MAY-22
Trichloroethylene			100.2		%		70-130	17-MAY-22
WG3728330-2	MB							
1,1,2,2-Tetrachloroethane			<0.50		ug/L		0.5	17-MAY-22
1,2-Dichlorobenzene			<0.50		ug/L		0.5	17-MAY-22
1,4-Dichlorobenzene			<0.50		ug/L		0.5	17-MAY-22
Benzene			<0.50		ug/L		0.5	17-MAY-22
Chloroform			<1.0		ug/L		1	17-MAY-22
cis-1,2-Dichloroethylene			<0.50		ug/L		0.5	17-MAY-22
Dichloromethane			<2.0		ug/L		2	17-MAY-22
Ethylbenzene			<0.50		ug/L		0.5	17-MAY-22
m+p-Xylenes			<0.40		ug/L		0.4	17-MAY-22
Methyl Ethyl Ketone			<20		ug/L		20	17-MAY-22
o-Xylene			<0.30		ug/L		0.3	17-MAY-22
Styrene			<0.50		ug/L		0.5	17-MAY-22
Tetrachloroethylene			<0.50		ug/L		0.5	17-MAY-22
Toluene			<0.40		ug/L		0.4	17-MAY-22
trans-1,3-Dichloropropene			<0.30		ug/L		0.3	17-MAY-22
Trichloroethylene			<0.50		ug/L		0.5	17-MAY-22
Surrogate: 1,4-Difluorobenzene			101.3		%		70-130	17-MAY-22
Surrogate: 4-Bromofluorobenzene			93.0		%		70-130	17-MAY-22
WG3728330-5	MS	WG3728330-3						
1,1,2,2-Tetrachloroethane			95.2		%		50-150	18-MAY-22
1,2-Dichlorobenzene			104.3		%		50-150	18-MAY-22
1,4-Dichlorobenzene			104.2		%		50-150	18-MAY-22
Benzene			92.5		%		50-150	18-MAY-22
Chloroform			108.5		%		50-150	18-MAY-22
cis-1,2-Dichloroethylene			107.3		%		50-150	18-MAY-22
Dichloromethane			113.1		%		50-150	18-MAY-22
Ethylbenzene			91.5		%		50-150	18-MAY-22
m+p-Xylenes			98.9		%		50-150	18-MAY-22
Methyl Ethyl Ketone			83.9		%		50-150	18-MAY-22



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Client: Grounded Engineering Inc
1 Banigan Drive
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Contact: Sam Baston

Test	Matrix	Reference	Result	Qualifier	Units	RPD	Limit	Analyzed
VOC-ROU-HS-WT								
	Water							
Batch	R5784216							
WG3728330-5 MS		WG3728330-3						
o-Xylene			92.7		%		50-150	18-MAY-22
Styrene			80.4		%		50-150	18-MAY-22
Tetrachloroethylene			91.2		%		50-150	18-MAY-22
Toluene			92.4		%		50-150	18-MAY-22
trans-1,3-Dichloropropene			78.2		%		50-150	18-MAY-22
Trichloroethylene			102.1		%		50-150	18-MAY-22

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Contact: Sam Baston

Legend:

Limit ALS Control Limit (Data Quality Objectives)
DUP Duplicate
RPD Relative Percent Difference
N/A Not Available
LCS Laboratory Control Sample
SRM Standard Reference Material
MS Matrix Spike
MSD Matrix Spike Duplicate
ADE Average Desorption Efficiency
MB Method Blank
IRM Internal Reference Material
CRM Certified Reference Material
CCV Continuing Calibration Verification
CVS Calibration Verification Standard
LCSD Laboratory Control Sample Duplicate

Sample Parameter Qualifier Definitions:

Qualifier	Description
J	Duplicate results and limits are expressed in terms of absolute difference.
K	Matrix Spike recovery outside ALS DQO due to sample matrix effects.
MES	Data Quality Objective was marginally exceeded (by < 10% absolute) for < 10% of analytes in a Multi-Element Scan / Multi-Parameter Scan (considered acceptable as per OMOE & CCME).
MS-B	Matrix Spike recovery could not be accurately calculated due to high analyte background in sample.
RPD-NA	Relative Percent Difference Not Available due to result(s) being less than detection limit.

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Contact: Sam Baston

Hold Time Exceedances:

ALS Product Description	Sample ID	Sampling Date	Date Processed	Rec. HT	Actual HT	Units	Qualifier
Physical Tests							
pH	1	12-MAY-22 10:00	17-MAY-22 00:00	4	5	days	EHT

Legend & Qualifier Definitions:

EHTR-FM: Exceeded ALS recommended hold time prior to sample receipt. Field Measurement recommended.
EHTR: Exceeded ALS recommended hold time prior to sample receipt.
EHTL: Exceeded ALS recommended hold time prior to analysis. Sample was received less than 24 hours prior to expiry.
EHT: Exceeded ALS recommended hold time prior to analysis.
Rec. HT: ALS recommended hold time (see units).

Notes*:
Where actual sampling date is not provided to ALS, the date (& time) of receipt is used for calculation purposes.
Where actual sampling time is not provided to ALS, the earlier of 12 noon on the sampling date or the time (& date) of receipt is used for calculation purposes. Samples for L2706126 were received on 12-MAY-22 16:30.

ALS recommended hold times may vary by province. They are assigned to meet known provincial and/or federal government requirements. In the absence of regulatory hold times, ALS establishes recommendations based on guidelines published by the US EPA, APHA Standard Methods, or Environment Canada (where available). For more information, please contact ALS.

The ALS Quality Control Report is provided to ALS clients upon request. ALS includes comprehensive QC checks with every analysis to ensure our high standards of quality are met. Each QC result has a known or expected target value, which is compared against pre-determined data quality objectives to provide confidence in the accuracy of associated test results.

Please note that this report may contain QC results from anonymous Sample Duplicates and Matrix Spikes that do not originate from this Work Order.

APPENDIX G



Equivalent Well Radius (TOWNHOUSE 1)

$$R_0 = 3000 \cdot dH \cdot K^{0.5}$$

$$r_s = (a+b)/3.14$$

applies when $a/b < 1.5$ and $R_0 \gg r_s$

$$r_s = ((a \cdot b)/3.14)^{0.5}$$

$$Q = \frac{3.14 \cdot K \cdot (H^2 - h_w^2)}{\ln(R_0/r_s)}$$

Ground Surface	99.85	masl
Highest Water Level	98.7	masl
Base of Excavation	96.55	masl
Drawdown Target	95.7	masl
Aquifer Bottom	95.7	masl
Rain Fall	0.030	m
Factor of Safety	2.0	
Hydraulic Gradient	1	
K =	1.00E-05	m/s
H =	3.0	m
h_w =	0.0	m
dH =	3.0	m
R_0 =	28.5	m
$r_s + R_0$ =	35	m
a =	12	m
b =	8	m
r_s =	6	m

SHORT TERM		
Summary	L/day	L/min
Groundwater	30,000	20.8
Rainfall	2,880	2.0
Total	32,880	22.8

100 year storm event (L/day):

10,000

LONG TERM		
Summary	L/day	L/min
Groundwater	30,000	20.8
Infiltration	3,000	2.1
Total	33,000	22.9

Legend:

K = Hydraulic Conductivity

H = Depth from static water table to the assumed aquifer bottom

h_w = Depth from the dewatering target to the assumed aquifer bottom

dH = Dewatering thickness

a = Length of Excavation

b = Width of Excavation

Reference:

J. Patrick Powers... [et al.] (2007), "Construction Dewatering and Groundwater Control: New Methods and Applications, 3rd ed." Wiley, Hoboken, NJ.

Equivalent Well Radius (TOWNHOUSE 2)

$$R_0 = 3000 \cdot dH \cdot K^{0.5}$$

$$r_s = (a+b)/3.14$$

applies when $a/b < 1.5$ and $R_0 \gg r_s$

$$r_s = ((a \cdot b)/3.14)^{0.5}$$

$$Q = \frac{3.14 \cdot K \cdot (H^2 - h_w^2)}{\ln(R_0/r_s)}$$

Ground Surface	99.85	masl
Highest Water Level	98.7	masl
Base of Excavation	96.55	masl
Drawdown Target	95.7	masl
Aquifer Bottom	95.7	masl
Rain Fall	0.030	m
Factor of Safety	2.0	
Hydraulic Gradient	1	
K =	1.00E-05	m/s
H =	3.0	m
h_w =	0.0	m
dH =	3.0	m
R_0 =	28.5	m
$r_s + R_0$ =	35	m
a =	12	m
b =	8	m
r_s =	6	m

SHORT TERM		
Summary	L/day	L/min
Groundwater	30,000	20.8
Rainfall	2,880	2.0
Total	32,880	22.8

100 year storm event (L/day):

10,000

LONG TERM		
Summary	L/day	L/min
Groundwater	30,000	20.8
Infiltration	3,000	2.1
Total	33,000	22.9

Legend:

K = Hydraulic Conductivity

H = Depth from static water table to the assumed aquifer bottom

h_w = Depth from the dewatering target to the assumed aquifer bottom

dH = Dewatering thickness

a = Length of Excavation

b = Width of Excavation

Reference:

J. Patrick Powers... [et al.] (2007), "Construction Dewatering and Groundwater Control: New Methods and Applications, 3rd ed." Wiley, Hoboken, NJ.

SHORT TERM - 1 BASEMENT LEVEL (TOWNHOUSE 3)			
Excavation Dimensions [m]		Rainfall Data	
N-S	8	Year	2
E-W	12	Hour	3
Area (m ²)	96	Depth (mm)	30
Perimeter (m)	40	Depth (m)	0.03
			0.098
Section	Flow [m³/day]	Length [m]	Volume [L/day]
Base	0	8	-
Sides	0	40	-
Total			-
Factor of Safety	2.0		-
Storm Events		Summary	L/day
2 Year [L/day]	100 Year [L/day]		L/min
2,880	10,000	Groundwater	-
		Rainfall	3,000
		Total	3,000
			2.1
			2.1

LONG TERM - 1 BASEMENT LEVEL (TOWNHOUSE 3)			
Excavation Dimensions [m]		Rainfall Data	
N-S	8	Year	2
E-W	12	Hour	3
Area (m ²)	96	Depth (mm)	30
Perimeter (m)	40	Depth (m)	0.03
			0.098
Section	Flow [m³/day]	Length [m]	Volume [L/day]
Base	0	8	-
Sides	0	40	-
Total			-
Factor of Safety	2.0		-
Infiltration [L/day]		Summary	L/day
1915.2			L/min
		Groundwater	-
		Infiltration	3,000
		Total	3,000
			2.1
			2.1

SHORT TERM - 1 BASEMENT LEVEL (TOWNHOUSE 4)			
Excavation Dimensions [m]		Rainfall Data	
N-S	8	Year	2
E-W	12	Hour	3
Area (m ²)	96	Depth (mm)	30
Perimeter (m)	40	Depth (m)	0.03
			0.098
Section		Flow [m³/day]	Length [m]
Base		0	8
Sides		0	40
Total			
Factor of Safety		2.0	
Storm Events		Summary	L/day
2 Year [L/day]	100 Year [L/day]		L/min
2,880	10,000	Groundwater	-
		Rainfall	3,000
		Total	3,000
			2.1
			2.1

LONG TERM - 1 BASEMENT LEVEL (TOWNHOUSE 4)			
Excavation Dimensions [m]		Rainfall Data	
N-S	8	Year	2
E-W	12	Hour	3
Area (m ²)	96	Depth (mm)	30
Perimeter (m)	40	Depth (m)	0.03
			0.098
Section		Flow [m³/day]	Length [m]
Base		0	8
Sides		0	40
Total			
Factor of Safety		2.0	
Infiltration [L/day]		Summary	L/day
1897.2			L/min
		Groundwater	-
		Infiltration	3,000
		Total	3,000
			2.1
			2.1

Equivalent Well Radius (TOWNHOUSE 5)

$$R_0 = 3000 \cdot dH \cdot K^{0.5}$$

$$r_s = (a+b)/3.14$$

applies when $a/b < 1.5$ and $R_0 \gg r_s$

$$r_s = ((a \cdot b)/3.14)^{0.5}$$

$$Q = \frac{3.14 \cdot K \cdot (H^2 - h_w^2)}{\ln(R_0/r_s)}$$

Ground Surface	96.22	masl
Highest Water Level	95.8	masl
Base of Excavation	92.92	masl
Drawdown Target	91.2	masl
Aquifer Bottom	80	masl
Rain Fall	0.030	m
Factor of Safety	2.0	
Hydraulic Gradient	1	
K =	7.21E-09	m/s
H =	15.8	m
h_w =	11.2	m
dH =	4.6	m
R_0 =	1.2	m
$r_s + R_0$ =	8	m
a =	12	m
b =	8	m
r_s =	6	m

SHORT TERM		
Summary	L/day	L/min
Groundwater	4,000	2.8
Rainfall	2,880	2.0
Total	6,880	4.8

100 year storm event (L/day):

10,000

LONG TERM		
Summary	L/day	L/min
Groundwater	4,000	2.8
Infiltration	3,000	2.1
Total	7,000	4.9

Legend:

K = Hydraulic Conductivity

H = Depth from static water table to the assumed aquifer bottom

h_w = Depth from the dewatering target to the assumed aquifer bottom

dH = Dewatering thickness

a = Length of Excavation

b = Width of Excavation

Reference:

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